RESEARCH AND DESIGN ASSISTED **BY TESTING**

lecture notes -

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2017

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- Malhotra V.M., CarinoN.J., Handbook on Nondestructive Testing of Concrete, CRC PRESS, 2004, ISBN 0-8031-2099-0
- Bungey J. H. , Millard S. G. , Grantham M. G., Testing of concrete in structures, Taylor & Francis, 2006, ISBN10: 0–415–26301–8
- Ryall M.J., Bridge Management, ISBN: 978-0-7506-8511-5
- Guidebook on non-destructive testing of concrete structures, IAEA, Vienna, 2002, IAEA– TCS–17, ISSN 1018–5518
- Newman J., Choo B. S., Advanced concrete technology, Oxford, England, 2003, ISBN 9780750656863
- SHM <u>http://www.shmlive.com/</u>
- SAMCO <u>http://www.samco.org/network/index.htm</u>
- ISIS and SAMCO Educational Module 5: An Introduction to Structural Health Monitoring



AIMS OF IN-SITU TESTING

1. CONTROL TESTING CARRIED OUT BY THE

- CONTRACTOR

OR

- CONCRETE PRODUCER

TO INDICATE ADJUSTMENTS NECESSARY TO ENSURE AN ACCEPTABLE SUPPLIED MATERIAL.

→ BULLETINS





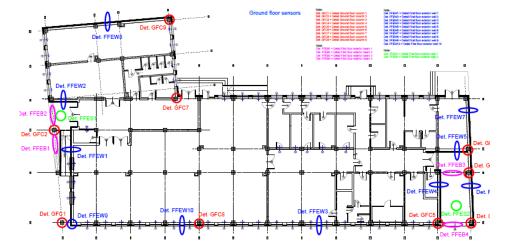
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AIMS OF IN-SITU TESTING

2. **COMPLIANCE TESTING** IS PERFORMED BY OR PERFORMED FOR

- THE ENGINEER ACCORDING TO AN AGREED PLAN

- TO JUDGE COMPLIANCE WITH THE SPECIFICATION



→ BULLETINS / REPORTS



(Bungey, 2006)

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AIMS OF IN-SITU TESTING

3. SECONDARY TESTING IS CARRIED OUT ON

- HARDENED CONCRETE IN THE STRUCTURE
- EXTRACTED FROM THE STRUCTURE.

THIS MAY BE REQUIRED WHEN:

- there is doubt about the reliability of control and compliance results
- they are unavailable results
- the results are inappropriate
- the structure is old, damaged or deteriorating

All testing which is **not planned before** construction will be in this category, although **long-term monitoring** is also included.

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(Bungey, 2006)

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NEED FOR TESTING IN/OF STRUCTURES

- PROPOSED CHANGE OF USAGE OR EXTENSION OF A STRUCTURE
- E. I. Factory \rightarrow store Store \rightarrow factory Residential building \rightarrow banc

Structural extensions: horizontal or vertical



- ACCEPTABILITY OF A STRUCTURE FOR **PURCHASE** OR **INSURANCE**



'BOOK OF THE CONSTRUCTION'



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NEED FOR TESTING IN/OF STRUCTURES

- MEMBERS KNOWN OR SUSPECTED TO CONTAIN **MATERIAL** WHICH DOES **NOT MEET SPECIFICATIONS**, OR WITH **DESIGN/EXECUTION FAULTS**



UNEVENNESS?



MORTAR CLASS ?

BRICK TYPE ACC/EFFICIENT ?



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NEED FOR TESTING IN/OF STRUCTURES

- **ASSESSMENT** OF THE **QUALITY** OR **INTEGRITY** OF APPLIED REPAIRS/STRENGTHENING

- MONITORING OF STRENGTH DEVELOPMENT IN RELATION TO CURING, PRESTRESSING OR LOAD APPLICATION

- MONITORING LONG-TERM CHANGES IN MATERIALS PROPERTIES AND STRUCTURAL PERFORMANCE

- **ASSESSMENT OF CAUSE** AND EXTENT **OF DETERIORATION** AS A PRELIMINARY TO THE DESIGN OF REPAIR OR REMEDIAL SCHEMES



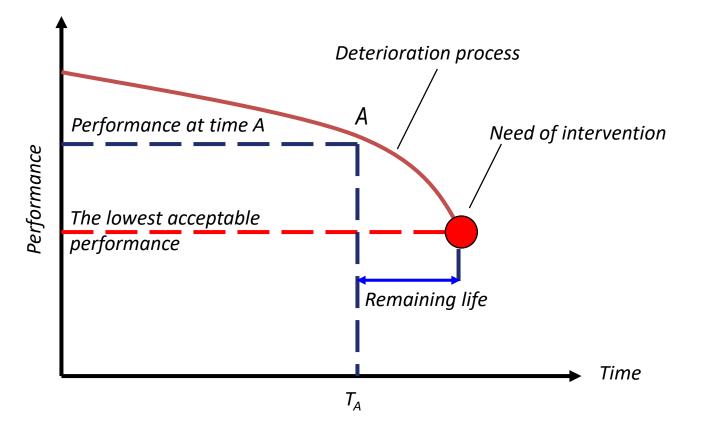


(Bungey, 2006)

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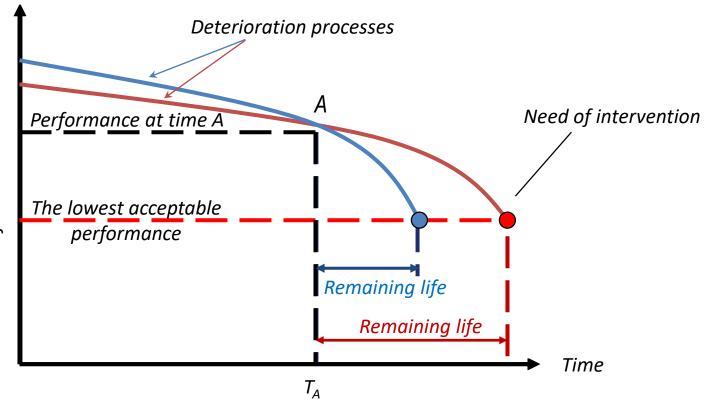
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DETERIORATION PROCESS IN GENERALLY



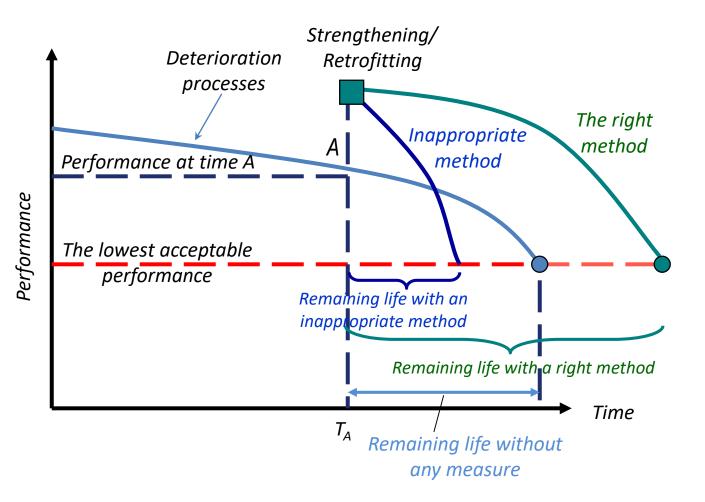


DETERIORATION PROCESS IN GENERALLY



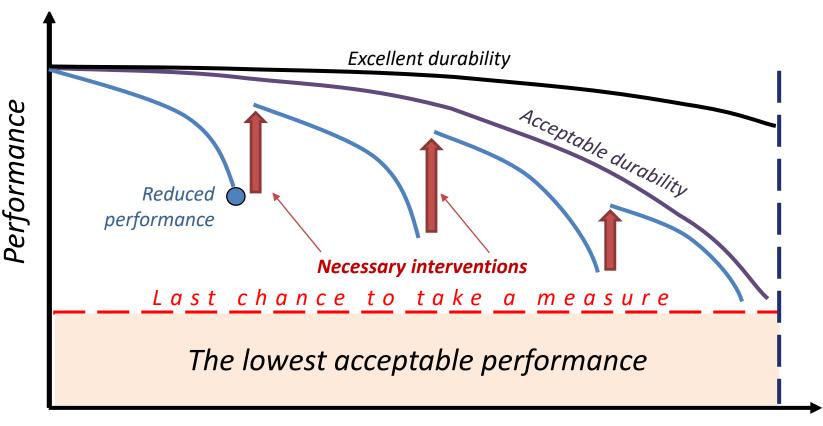


DETERIORATION PROCESS IN GENERALLY – IMPORTANCE OF A PROPER INTERVENTION





DETERIORATION PROCESS IN GENERALLY



Age of the structure



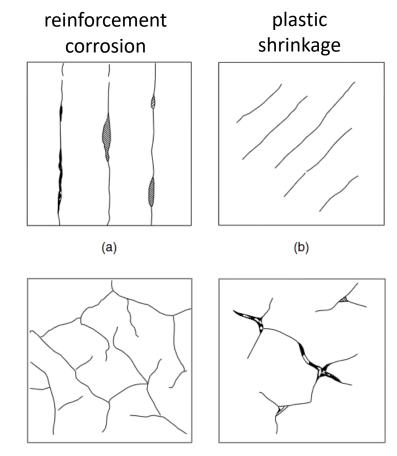
TYPE OF INVESTIGATIONS

INVESTIGATIONS ON MATERIALS

- DETERMINATIONS ON STRENGTHS
- PHYSICAL DETERMINATIONS
- CHEMICAL DETERMINATIONS
- OTHER DETERMINATIONS

INVESTIGATIONS ON STRUCTURES

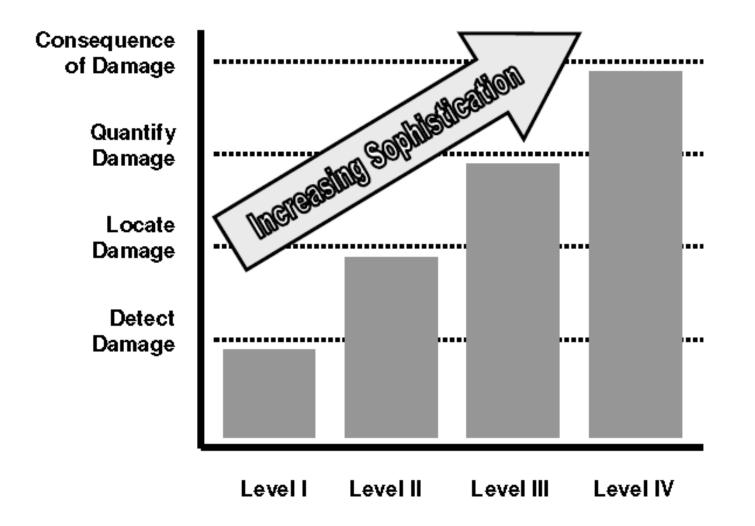
- STUDY OF CRACKING OR CRACKS
- DEFORMATION MEASURING
- STRESS MEASURING
- STRUCTURAL INTEGRITY
- OTHER DETERMINATIONS



^(c) sulfate attack

^(d) alkali/aggregate reaction

INVESTIGATION LEVELS



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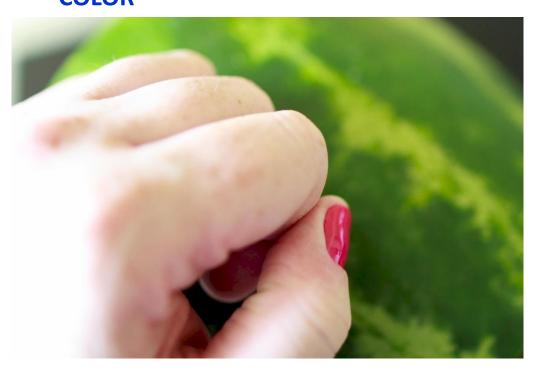
(ISIS, 2006)

RESEARCH AND DESIGN ASSISTED BY TESTING

MODES OF INVESTIGATION INSTINCTIVE



- SURFACE HARDNESS - COLOR





RESEARCH AND DESIGN ASSISTED BY TESTING

MODES OF INVESTIGATION

INSTINCTIVE



- CORE: COLOR
 - ASPECT
 - **DENSITY**
 - SMELL
 - HARDNESS
 - COMPACTNESS



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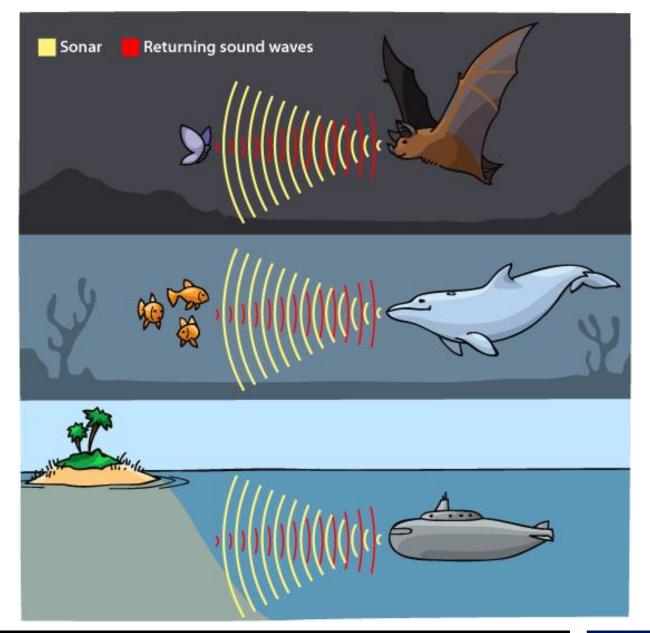




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INSTINCTIVE

- **DISTANCE**
- NUMBER
- SIZE
- POSITION

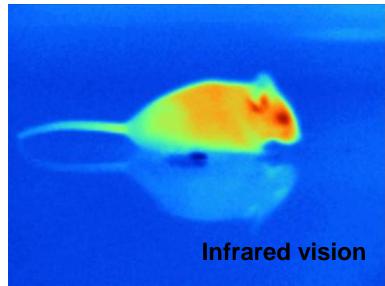




MODES OF INVESTIGATION

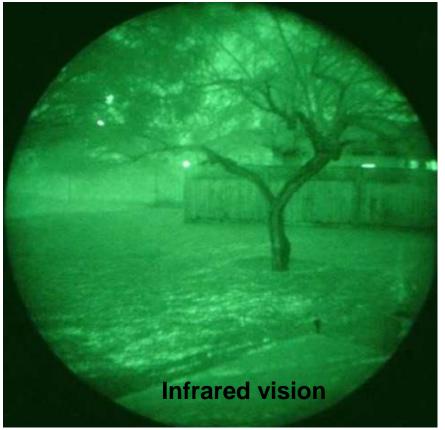
INSTINCTIVE





- **TEMPERATURE**
- SIZE
- NUMBER





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AVAILABLE TEST METHODS

- NON-DESTRUCTIVE METHODS, DEFINED AS NOT IMPAIRING THE PERFORMANCE OF THE ELEMENT OR MEMBER UNDER TEST, AND WHEN APPLIED TO CONCRETE IS TAKEN TO INCLUDE METHODS WHICH CAUSE LOCALIZED SURFACE ZONE DAMAGE

- METHODS REQUIRING SAMPLE EXTRACTION, MOST COMMONLY IN THE FORM OF CORES DRILLED FROM THE CONCRETE, WHICH MAY BE USED IN THE LABORATORY FOR STRENGTH AND OTHER PHYSICAL TESTS AS WELL AS VISUAL, PETROGRAPHIC (MINERAL CONTENT) AND CHEMICAL ANALYSIS.

(Bungey, 2006)

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PRINCIPAL TEST METHODS

INVESTIGATION	TEST	
CORROSION OF	COVER DEPTH	
EMBEDDED STEEL	CARBONATION DEPTH	
	CHLORIDE CONCENTRATION	
	RESISTIVITY	

(Bungey, 2006)

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INVESTIGATION	TEST	
CONCRETE QUALITY	SURFACE HARDNESS	
AND DURABILITY	ULTRASONIC PULSE VELOCITY	
	RADIOGRAPHY / RADIOMETRY	
	RELATIVE HUMIDITY	
	PERMEABILITY	
	ABSORPTION	
	SULFATE CONTENT	
	PETROGRAPHIC (MINERAL CONTENT)	
	AIR CONTENT	
	CEMENT TYPE AND CONTENT	
	ABRASION RESISTANCE	

(Bungey, 2006)

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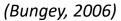
INVESTIGATION	TEST
CONCRETE STRENGTH	CORES
	PULL-OUT
	PULL-OFF
	PENETRATION RESISTANCE
	MATURITY

(Bungey, 2006)

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INVESTIGATION	TEST
INTEGRITY AND PERFORMANCE	DYNAMIC RESPONSE
	THERMOGRAPHY
	RADAR
	REINFORCEMENT LOCATION
	STRAIN OR CRACK MEASUREMENT
	LOAD TEST



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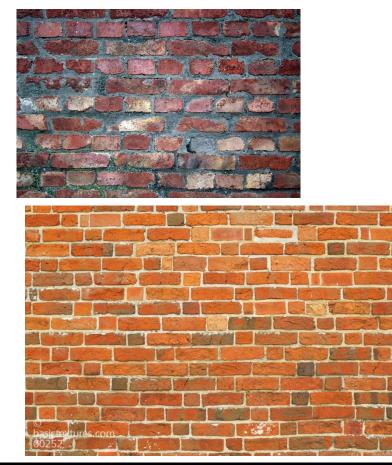
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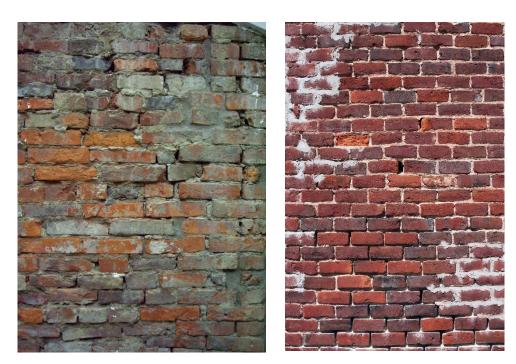
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 \rightarrow This **can** often **provide valuable information** to the well-trained eye \odot :

- QUALITY OF WORKS – verticality, color, thickness, width, solid, compact, deformed





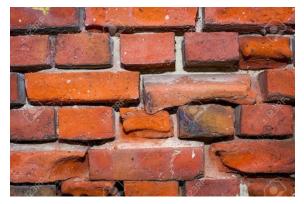
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 \rightarrow This can often provide valuable information to the well-trained eye \odot :

- QUALITY OF MATERIALS







 \rightarrow This **can** often **provide valuable information** to the well-trained eye \odot :

- SEGREGATION / HONEYCOMBS





Honeycombs : are the hollow spaces and cavities left in concrete mass on surface or inside the concrete mass which is caused by the mortar not filling the spaces between the coarse aggregate particles.

Causes: poor workability, poor grading of aggregates, grout leak, movement of formwork, improper compaction, improper cover and placement of rebars.

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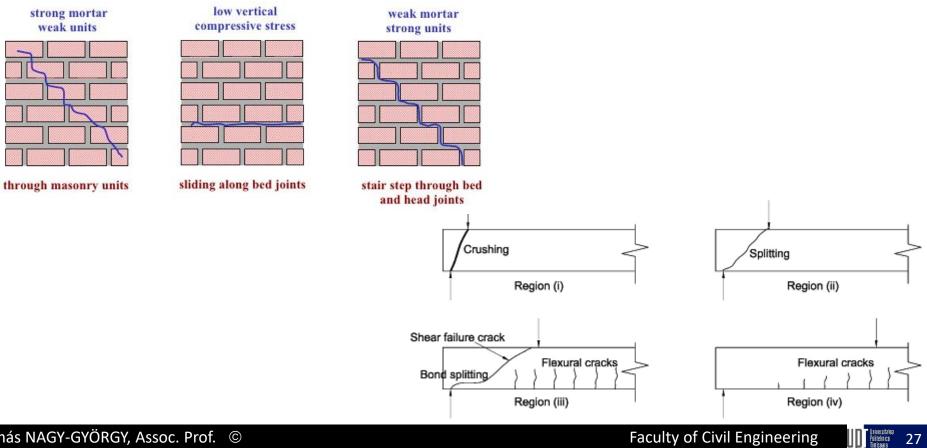


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VISUAL INSPECTION

- \rightarrow This can often provide valuable information to the well-trained eye \odot :
- VARIOUS TYPES OF CRACKING (SHRINKAGE, CORROSION, BENDING, EXCESSIVE

DEFORMATION/DEFLECTIONS)



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 \rightarrow This can often provide valuable information to the well-trained eye \odot :

- SPALLING OF CONCRETE from fire, freeze-thaw, rebar corrosion, stresses

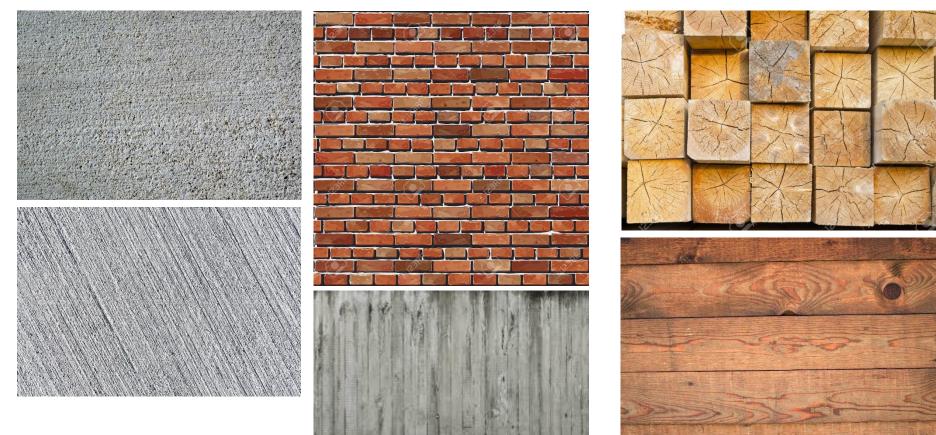


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 \rightarrow This **can** often **provide valuable information** to the well-trained eye \bigcirc :

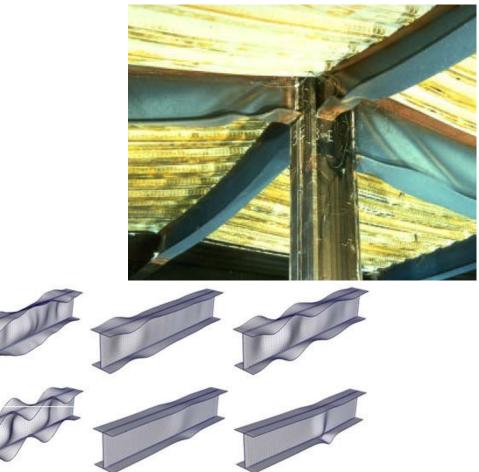
- UNIFORMITY OF CONCRETE SURFACE TEXTURE AND COLOR VARIATIONS



 \rightarrow This **can** often **provide valuable information** to the well-trained eye \odot :

- STRUCTURAL DEFORMATIONS – deflections, buckling, rotations







DIAGNOSIS OF DEFECTS AND DETERIORATION

(Bungey, 2006)

DIAGNOSIS OF DEFECTS AND DETERIORATION

CAUSE	SYMPTOMS			AGE OF APPEARANCE	
	cracking	spalling	erosion	early	long-term
REINFORCEMENT CORROSION	X	Х			Х
CHEMICAL ATTACK	X	Х	Х		X
FROST DAMAGE	X	Х	Х	Х	
FIRE DAMAGE	X	Х		Х	
FREEZE-THAW		Х	Х		X
INTERNAL REACTIONS	X	Х			X
THERMAL EFFECTS	X	Х		Х	X
SHRINKAGE	X			Х	X
CREEP	Х	Х			Х
SETTLEMENT	Х			Х	Х
PHYSICAL DAMAGE	X	Х	Х	Х	Х
STRUCTURAL DEFICIENCY	X	Х		Х	X

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(Bungey, 2006)

DURABILITY TESTS \rightarrow CHARACTERISTICS

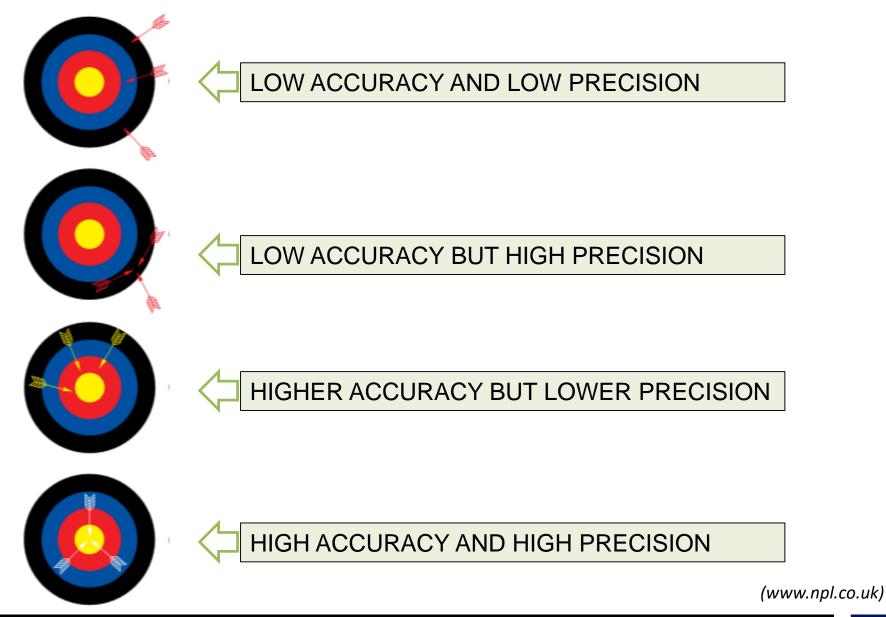
METHOD	COST	SPEED OF TEST	DAMAGE	
COVER MEASUREMENT	LOW	FAST	NONE	
CARBONATION DEPTH	LOW	FAST	MINOR	
CHLORIDE CONTENT	LOW	FAST	MINOR	
RESISTIVITY	MODERATE	FAST	MINOR/NONE	
ABSORPTION	MODERATE	SLOW	MODERATE/MINOR	
PERMEABILITY	MODERATE	SLOW	MODERATE/MINOR	
MOISTURE CONTENT	MODERATE	SLOW	MINOR	
CHEMICAL	MODERATE/HIGH	SLOW	MODERATE	
RADIOGRAPHY	HIGH	SLOW	NONE	
PETROGRAPHIC	HIGH	SLOW	MODERATE	



STRENGTH TESTS \rightarrow CHARACTERISTICS

METHOD	соѕт	SPEED OF TEST	DAMAGE	REPRESEN- TATIVENESS	RELIABILITY OF ABSOLUTE STRENGTH CORRELATIONS
CORES	HIGH	SLOW	MODERATE	MODERATE	GOOD
PULL-OUT	MODERATE	FAST	MINOR	NEAR SURFACE ONLY	MODERATE
PENETRATION	MODERATE	FAST	MINOR	NEAR SURFACE ONLY	MODERATE
PULL-OFF	MODERATE	FAST	MINOR	NEAR SURFACE ONLY	MODERATE
ULTRASONIC PULSE VELOCITY	LOW	FAST	NON	GOOD	POOR
SURFACE HARDNESS	VERY LOW	FAST	UNLIKELY	SURFACE ONLY	POOR
MATURITY	MODERATE	CONTINUOUS	VERY MINOR	GOOD	MODERATE

COMMENTS



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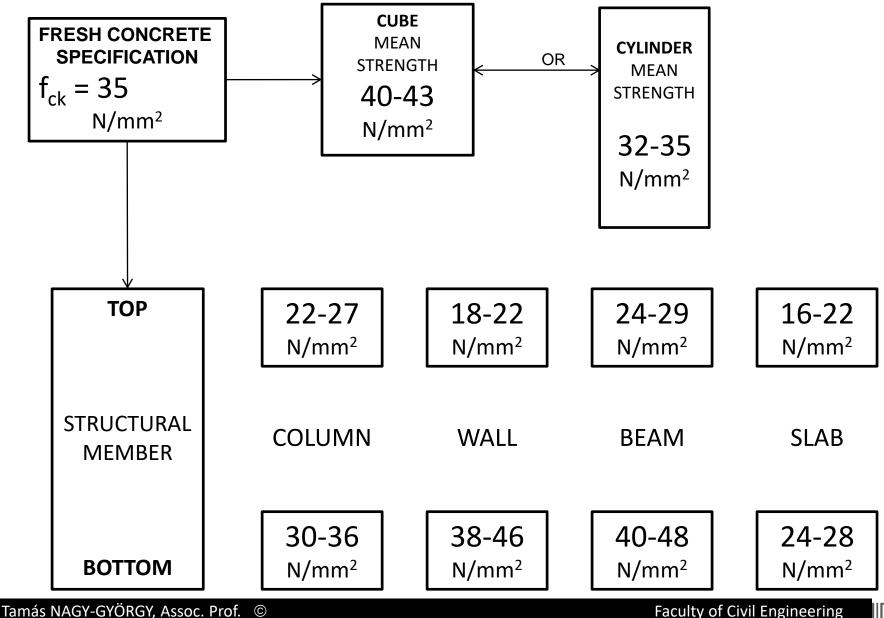
NUMBER OF TESTS

TEST METHOD	NO. OF INDIVIDUAL TEST RECOMMENDED AT A LOCATION
'STANDARD' CORES	3
SMALL CORES	9
SCHMIDT HAMMER	12
ULTRASONIC PULSE VELOCITY	9
PULL-OUT	4
WINDSOR PROBE	3
PULL-OFF	6



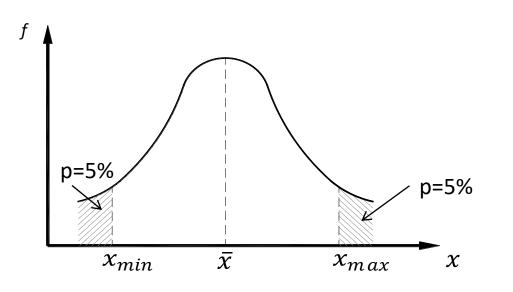
(Bungey, 2006)

TYPICAL RELATIONSHIP BETWEEN STANDARD SPECIMEN AND IN-SITU STRENGTH

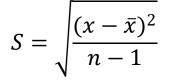


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ACCURACY OF TEST RESULTS



$$c_{\nu} = 100 \frac{S}{\bar{x}}$$



$$\bar{x} = \frac{\Sigma x}{n}$$

$$x_{min\,/max} = \bar{x} \pm tS = \bar{x}(1 \pm t \cdot c_v)$$

t = f(n, p%)

- S standard deviation
- \overline{x} mean value
- *c*_v *coefficient of variation (COV)*
- t coefficient

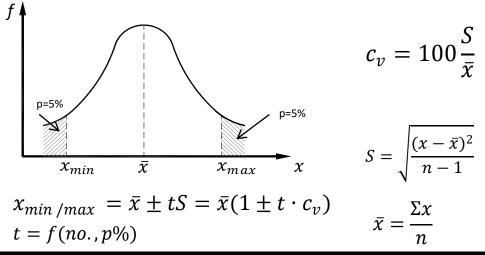
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Test method	Typical COV for individual member of good quality construction (in %)	Best 95% confidence limits on strength estimates (in %)
Cores – standard	10	\pm 10 (3 specimens)
– small	15	± 15 (9 specimens)
Pull-out	8	± 20 (4 tests)
Internal fracture	16	± 28 (6 tests)
Pull-off	8	± 15 (6 tests)
Break-off	9	± 20 (5 tests)
Windsor probe	4	± 20 (3 tests)
Ultrasonic pulse velocity	2.5	± 20 (l test)
Rebound hammer	4	± 25 (12 tests)

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(Bungey, 2006)

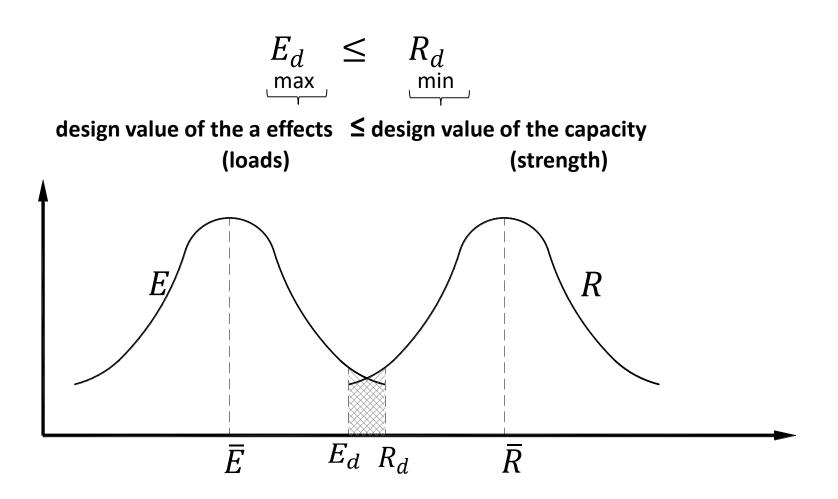
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Rebound hammer	4	± 25 (12 tests)



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In ULS general condition for capacity test is :

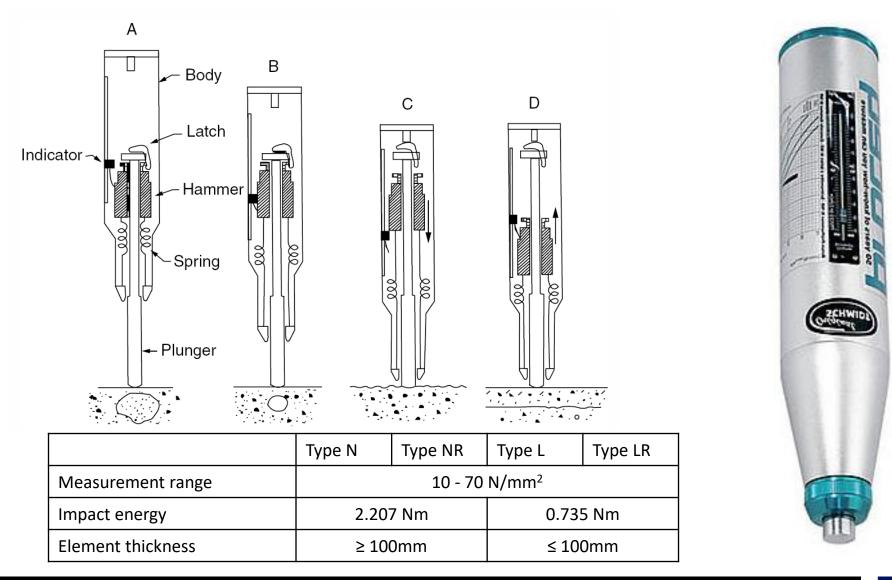


5% x 5% = 2.5‰ \rightarrow from 2000 elements could be considered <u>failure</u> of 5 elemente



(Malhotra, 2004)

1. SURFACE HARDNESS METHOD USING SCHMIDT REBOUND HAMMER



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1. SURFACE HARDNESS METHOD USING SCHMIDT REBOUND HAMMER

- \rightarrow true rebound value and excellent repeatability
- \rightarrow high measurement accuracy
- \rightarrow The rebound value requires no angular correction





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(Proceq)

1. SURFACE HARDNESS METHOD USING SCHMIDT REBOUND HAMMER

The results of the Schmidt rebound hammer are affected by:

- 1. Smoothness of test surface
- 2. Size, shape, and rigidity of the specimens
- 3. Age of test specimens
- 4. Surface and internal moisture conditions of the concrete
- 5. Type of coarse aggregate
- 6. Type of cement
- 7. Type of mold
- 8. Carbonation of the concrete surface
- 9. Stress state and temperature



1. SURFACE HARDNESS METHOD USING SCHMIDT REBOUND HAMMER

Applications and limitations:

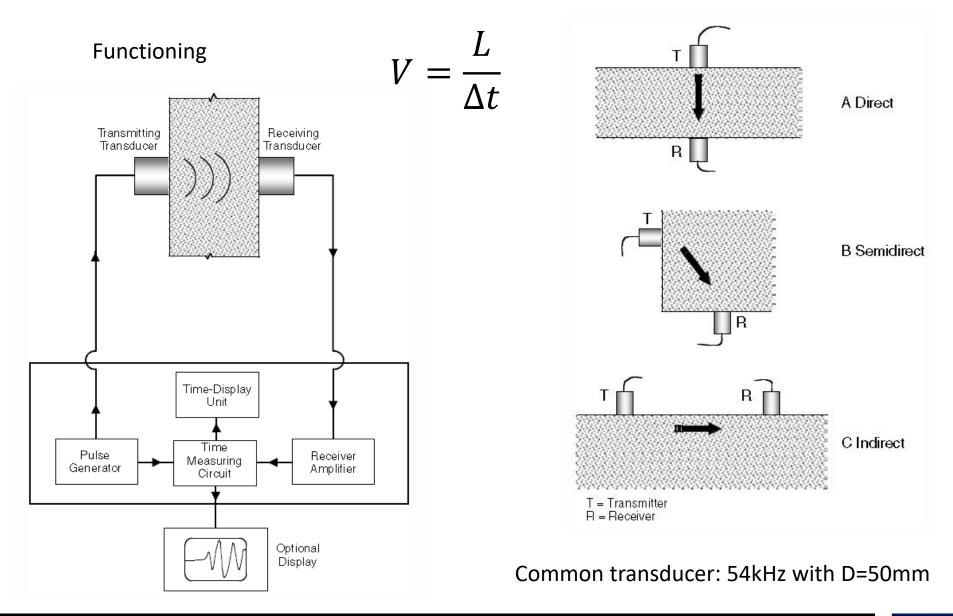
- 1. Checking the uniformity of concrete quality
- 2. Comparing a given concrete with a specified requirement
- 3. Approximate estimation of strength
- 4. Abrasion resistance classification.



NON-DESTRUCTIVE TESTING OF CONCRETE

(*Malhotra*, 2004)

2. THE ULTRASONIC PULSE VELOCITY METHOD



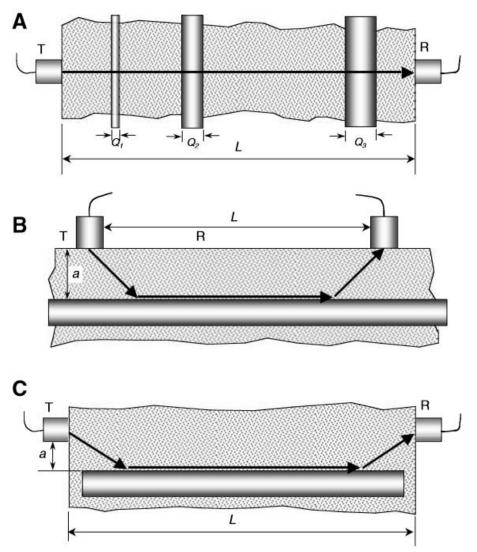
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(*Malhotra*, 2004)

2. THE ULTRASONIC PULSE VELOCITY METHOD



The pulse velocity measured in **reinforced concrete in the vicinity of reinforcing bars is often higher than in plain concrete** of the same composition. This is because the compressional pulse velocity in steel is 1.4 to 1.7 times that in plain concrete, the first pulse to arrive at the receiving transducer travels partly in concrete and partly in steel.

The increase in pulse velocity depends on:

- concrete cover
- dimensions and no. of the reinforcing bars
- bars orientation with respect to the propagation path
- pulse velocity in the surrounding concrete



NON-DESTRUCTIVE TESTING OF CONCRETE

(Proceq, 2011)

2. THE ULTRASONIC PULSE VELOCITY METHOD



Cement type: P for Portland cement B for blast furnace cement

Correction factor for moisture, rebar vicinity or other influences

Rebound value inserted

Measurement number

Transmission time of the sound waves between the transducers in $\ensuremath{\mu s}$

Distance between the transducers, unit preselectable: m, ft

· Pulse velocity v= l/t

Concrete strength TICO-SCHMIDT, unit preselectable N/mm², MPa, kg/cm², psi

Instructions for operation



2. THE ULTRASONIC PULSE VELOCITY METHOD

Factors affecting pulse velocity:

- 1. Aggregate Size, Grading, Type, and Content
- 2. Cement Type, Water–Cement Ratio, Admixtures
- 3. Age of Concrete
- 4. Transducer Contact
- 5. Temperature of Concrete
- 6. Moisture and Curing Condition of Concrete
- 7. Path Length
- 8. Size and Shape of a Specimen
- 9. Level of Stress, Stress history
- 10.Presence of Reinforcing Steel :
- Axis of bars parallel to pulse path
- Axis of bars perpendicular to the pulse path



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NON-DESTRUCTIVE TESTING OF CONCRETE

(Malhotra, 2004)

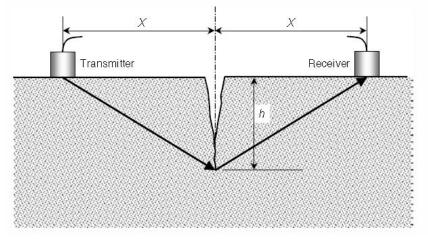
2. THE ULTRASONIC PULSE VELOCITY METHOD

Laboratory applications:

- 1. Monitoring of experiments strength development
 - deterioration
 - cracks

In-situ applications:

- 1. Measurement of concrete uniformity
- 2. Strength estimation
- 3. Detection of cracking and honeycombing
- 4. Assessment of concrete deterioration
- 5. Measurement of layer thickness
- 6. Measurement of elastic modulus
- 7. Strength development monitoring
- 8. Ultrasonic Imaging



$$h = \frac{x}{T_2} \sqrt{T_1^2 - T_2^2}$$

- $\mathbf{x}-\mathbf{distance}$ to the transducer from the crack
- $T_1-\mbox{transit time}$ around the crack

 $\rm T_2-transit time along the surface of the same type of concrete without any crack$

Accuracy: $\pm 15-20\% \rightarrow$ if the composition is known + probes $\pm 20-30\% \rightarrow$ if the composition is unknown $\pm 30-40\% \rightarrow$ missing information



3. COMBINED METHODS

Combination of the methods:- Ultrasonic Pulse Velocity

 Hardness Measurement Techniques using rebound (Schmidt) hammer

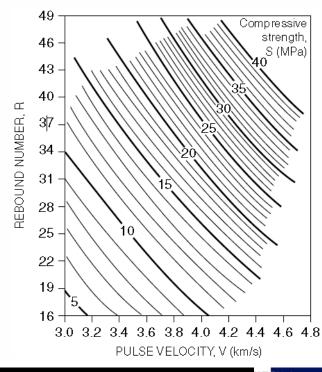
Advantages:

- higher accuracy than a simple method
- not need to know maturity of concrete
- less influence by the variation in cement dosage or cement type, or

Applications :

- 1. Evaluation the in situ properties of concrete
- 2. Determination of concrete uniformity
- 3. Determination grade of compaction

Accuracy:±10-15% → if the composition is known + probes ±15-20% → if the composition is known ±25-35% → missing information (I. Făcăoaru)



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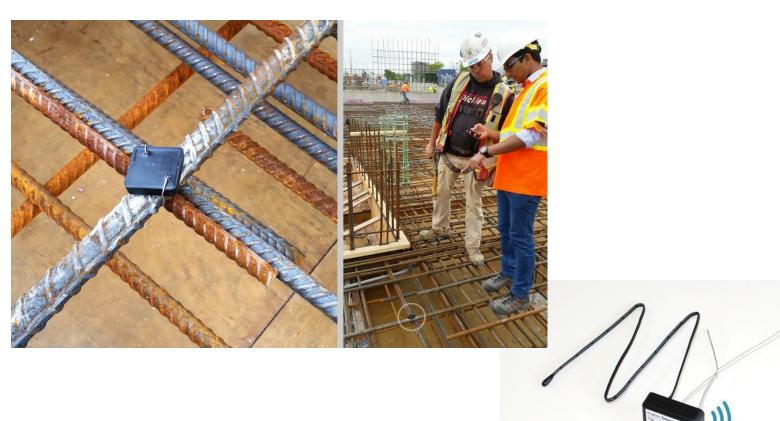
Waterproof wireless sensor for monitoring the temperature of concrete from fresh stage to hardened stage

\rightarrow maturity-based strength estimation of concrete

Real time monitoring of temperature \rightarrow

- optimizing the formwork removal time
- application of load on the structure
- adjusting the curing temperature of concrete onsite (heating and cooling processes)
- laboratory tests schedule





SmartRock™ Wireless Concrete Sensor

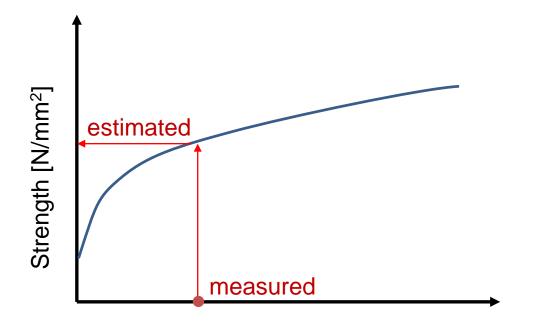
(Giatec Scientific)

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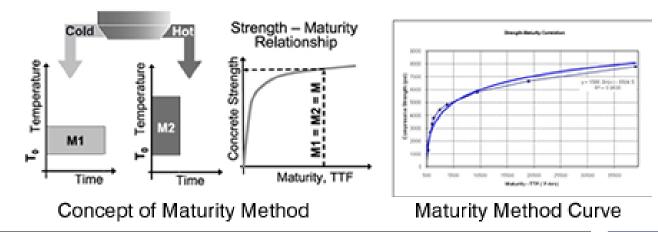
33.0 Last strength (Mar) 18.1 Last Maturity (Cchra) 726.5

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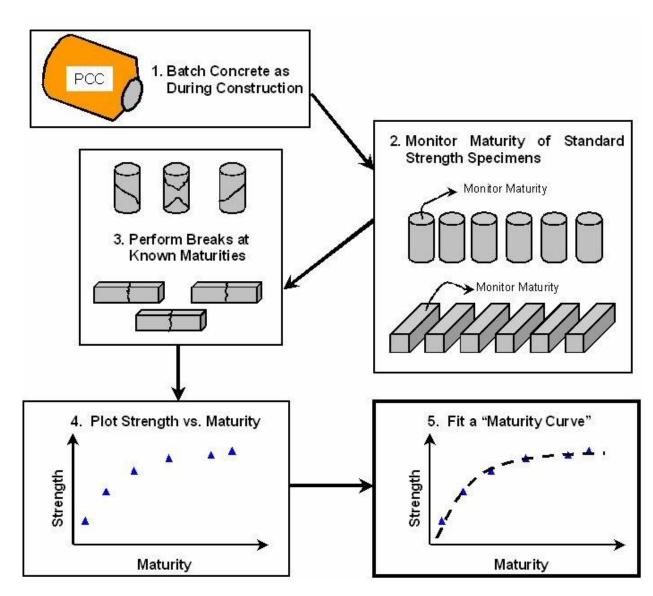
Maturitaty (°C x time)



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(www.worldoftest.com)

4. PENETRATION RESISTANCE METHOD → WINDSOR PROBE



Windsor HP Probe - ASTM C-803

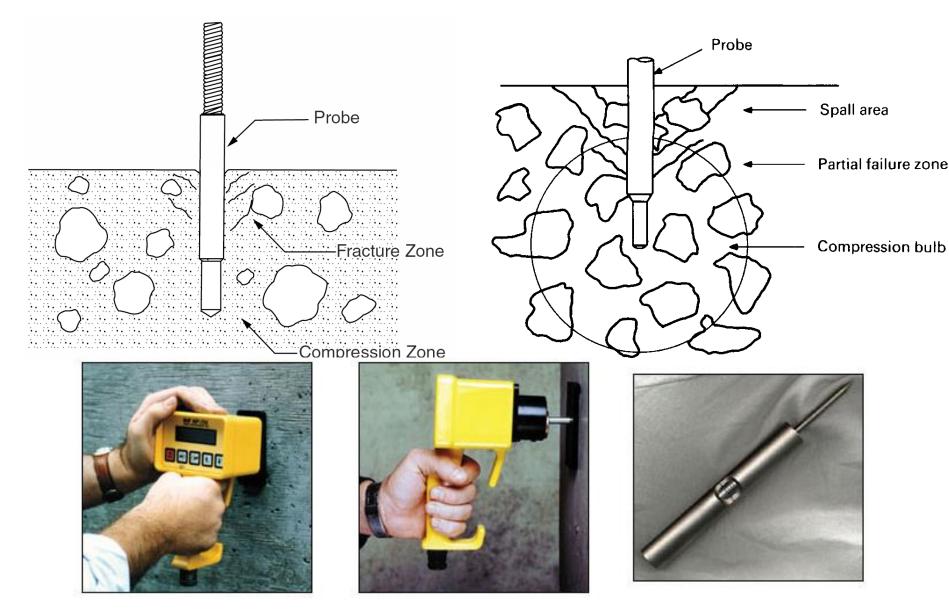






(Bungey, 2006)

4. PENETRATION RESISTANCE METHOD \rightarrow WINDSOR PROBE



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(Hilti, 2011)

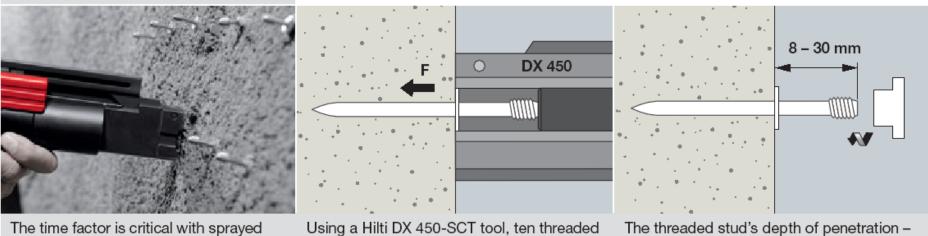
4. PENETRATION RESISTANCE METHOD → Hilti DX 450-SCT

SCT = Sprayed Concrete Testing









The time factor is critical with sprayed

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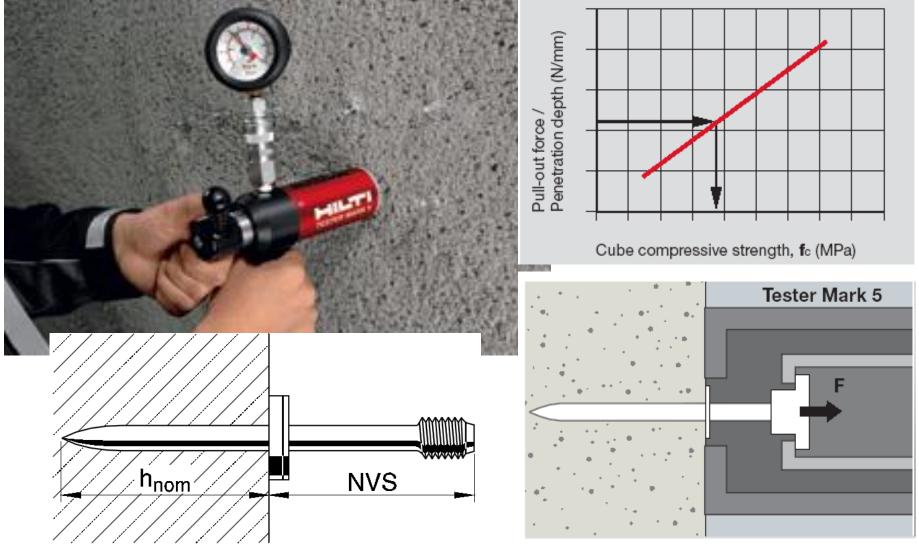
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(Hilti, 2011)

4. PENETRATION RESISTANCE METHOD → Hilti DX 450-SCT

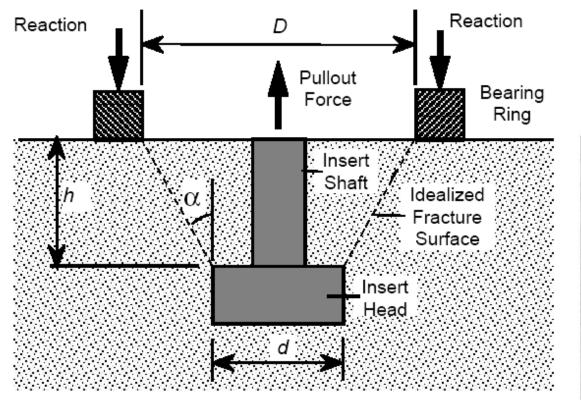
SCT = Sprayed Concrete Testing

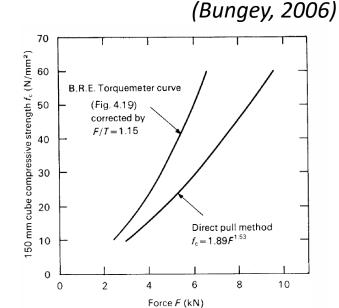


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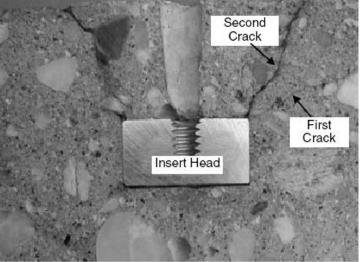
5. PULL-OUT TEST METHOD

2 categories: a) cast-in-place pull-out test





(*Malhotra*, 2004)

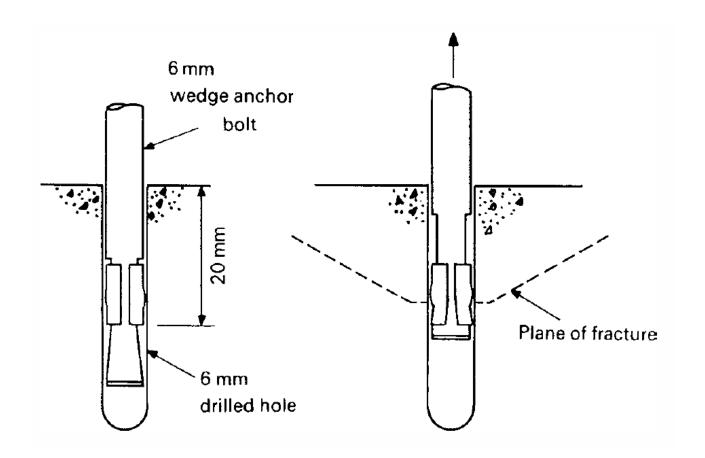


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(Bungey, 2006)

5. PULL-OUT TEST METHOD

b) Post-Installed pull-out test

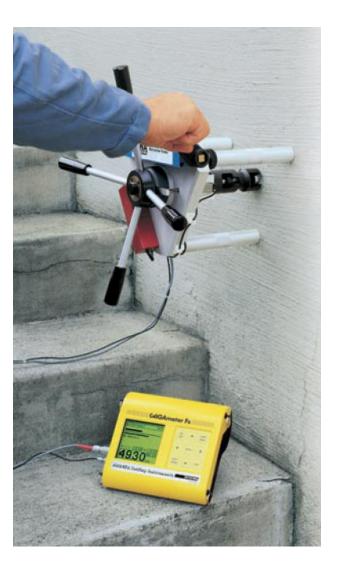




(Proceq, 2011)

5. PULL-OUT TEST METHOD \rightarrow equipment

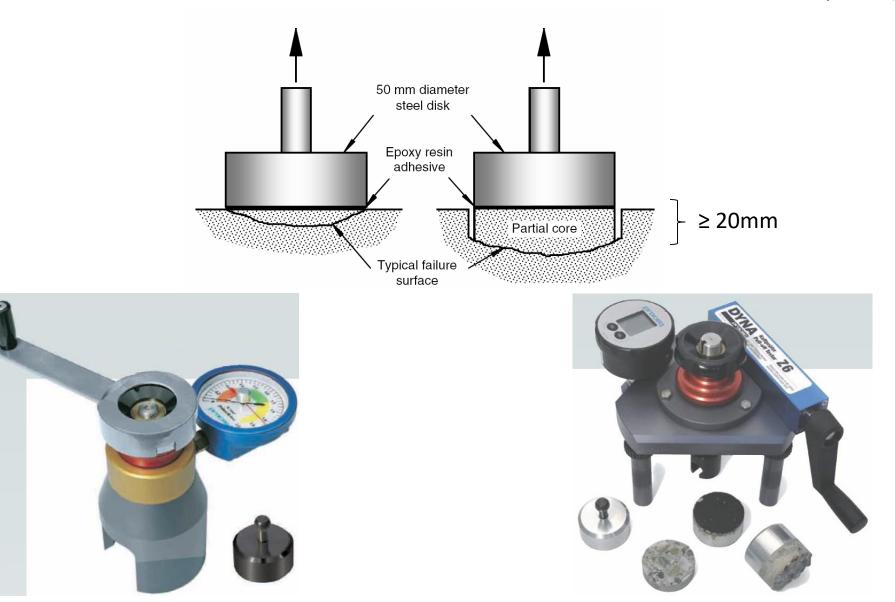






6. PULL-OFF TEST METHOD

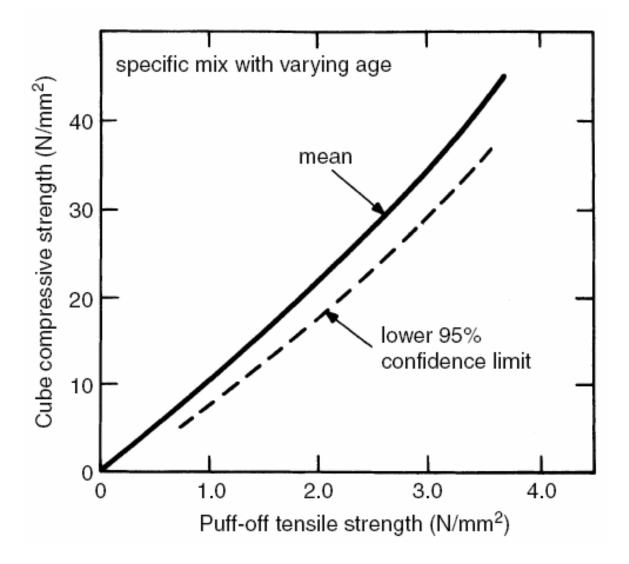
(Malhotra, 2004; Proceq, 2011)





(Bungey, 2006)

6. PULL-OFF TEST METHOD





Diameter and height of the cores by different standards:

- D > 3 x d_{max,aggreg} (British and American standards)
- Dmin = 100mm, Dopt = 150mm
- Accuracy decrese if dmax,aggreg / Dcore increse
- If $d_{max, aggreg} > 25mm \rightarrow D_{min} > 100mm$
- H/D = 1.0 ÷ 2.0



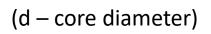
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63

NP 137 - 2014

- No of cores \rightarrow n = 42/d
- Core diameter \rightarrow $d \ge 3\phi_{max}$
- Minim diameter \rightarrow d_{min} = 50 mm
- Recommended diameter $\rightarrow d_{opt} = 100 \text{ mm}$
- Core height \rightarrow d \leq h \leq 2d



 $(\phi_{max} - maxim aggregate diameter)$

(recommended by SR EN 13791)





1) Visual inspection :

- type, size and distribution of the aggregates
- holes, defects, cracks
- compaction
- 2) Cutting and edge correction
- 3) Determination of the density
- 4) Determination of the compression strength
 - Variables: H/D
 - D
 - coring direction
 - correction method and material



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- intersected reinforcements



JL2.2

131

132

JL3.3

JL11

JL1.2.

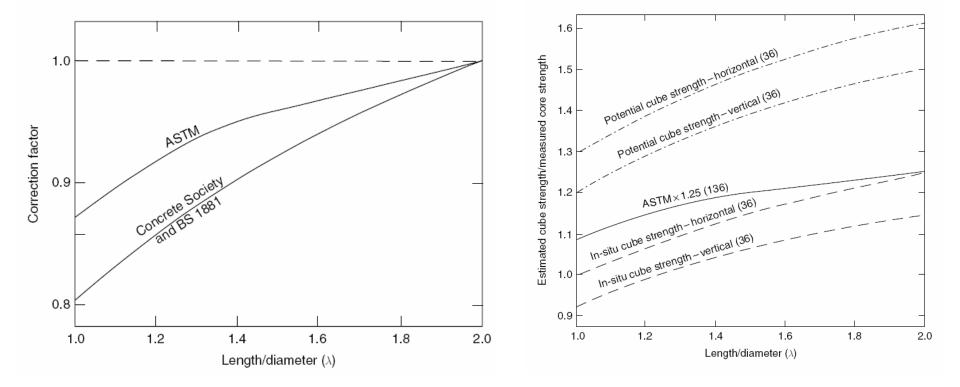


65

T151

JL 42 115.2

Result corrections and interpretations

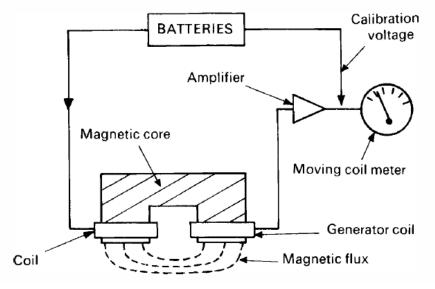




DURABILITY TESTS

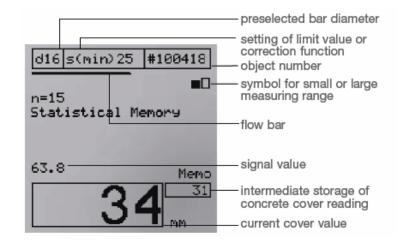
(Proceq, 2011)

8. DETERMINATION OF CONCRETE COVER DEPTH AND REBAR LOCATION





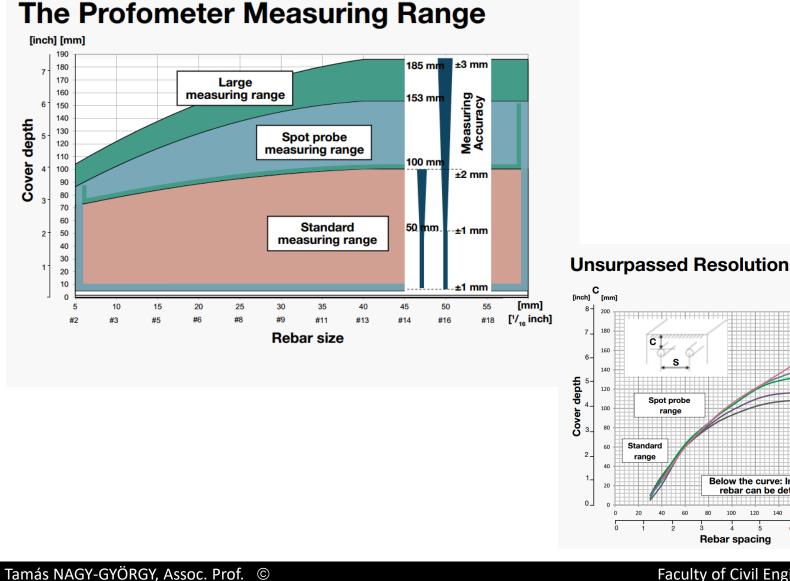




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(*Proceq*, 2011)

8. DETERMINATION OF CONCRETE COVER DEPTH AND REBAR LOCATION



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100 120 140

Rebar spacing

Below the curve: Individual rebar can be detected

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#13

12

#8

#4

#3

#2

Ø 40 mm

Ø 26 mm

Ø 12 mm

Ø 8 mm

S^[mm]

[inch]

(Proceg, 2011)

200

180 160

Ø 16 mm # 5

DURABILITY TESTS

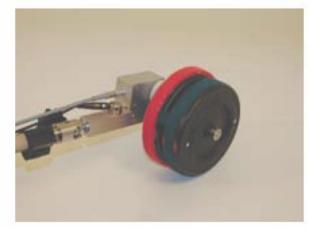
(Proceq, 2011)

9. DETERMINATION OF THE REINFORCEMENT CORROSION

→ CORROSION ANALYSIS WITH THE HALF-CELL POTENTIAL METHOD: ACCURATE FIELD POTENTIAL MEASUREMENTS AID IN DETECTING ACTIVE CORROSION IN REBARS



Standard half cell rod electrode



Wheel electrode with moistening wheel for continuous wetting up to a length of 200m (650ft). Linear distance recorder with travel direction detection. Automatic measurement at pre-selected intervals.



Four-point Wenner probe



DURABILITY TESTS

10. MOISTURE MEASUREMENT









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11. ABSORPTION AND PERMEABILITY TESTS



 \rightarrow two-chamber vacuum cell and a pressure regulator, which ensure that an air flow at right angles to the surface is directed towards the inner chamber. This permits the calculation of the permeability coefficient kT on the basis of a simple theoretical model



11. TESTS FOR FREEZE–THAW RESISTANCE





(Schleibinger Testing Systems)

(brickandtile.org)

The test specimens are placed in a solution of sodium chloride (NaCl) to stress the surface. The result gives an estimate for the resistance to freeze-thaw cycling for the concrete being tested.

In connection with de-icing salt, the frost attack is considerably stronger.

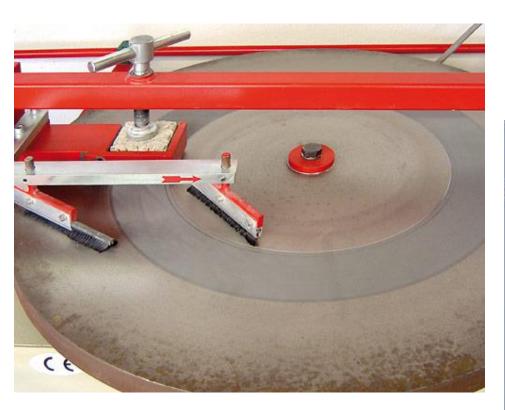
EN 12390-9

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10. ABRASION RESISTANCE TESTING







10. ABRASION RESISTANCE TESTING

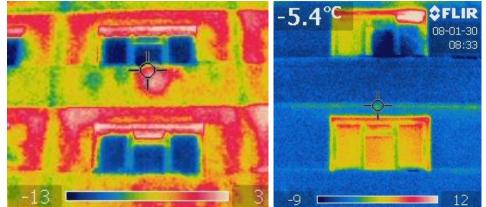




11. INFRARED THERMOGRAPHY







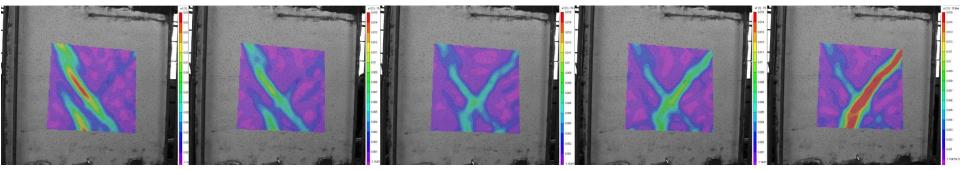
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12. OPTICAL DEFORMATION MEASUREMENT TECHNIQUE



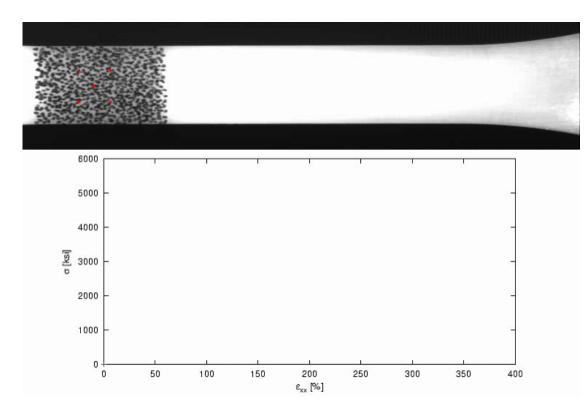




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12. OPTICAL DEFORMATION MEASUREMENT TECHNIQUE

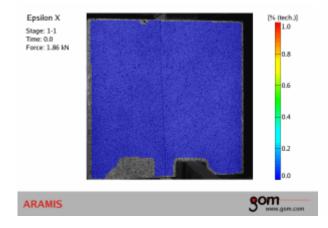




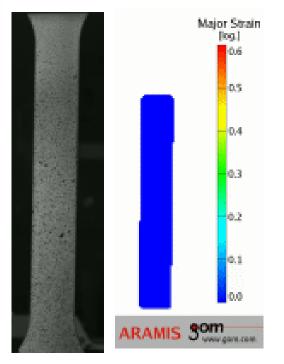




12. OPTICAL DEFORMATION MEASUREMENT TECHNIQUE



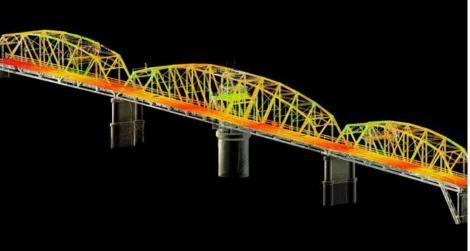






13. 3D LASER SCANNING



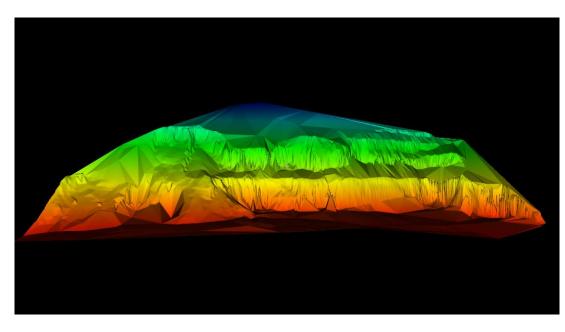








13. 3D LASER SCANNING



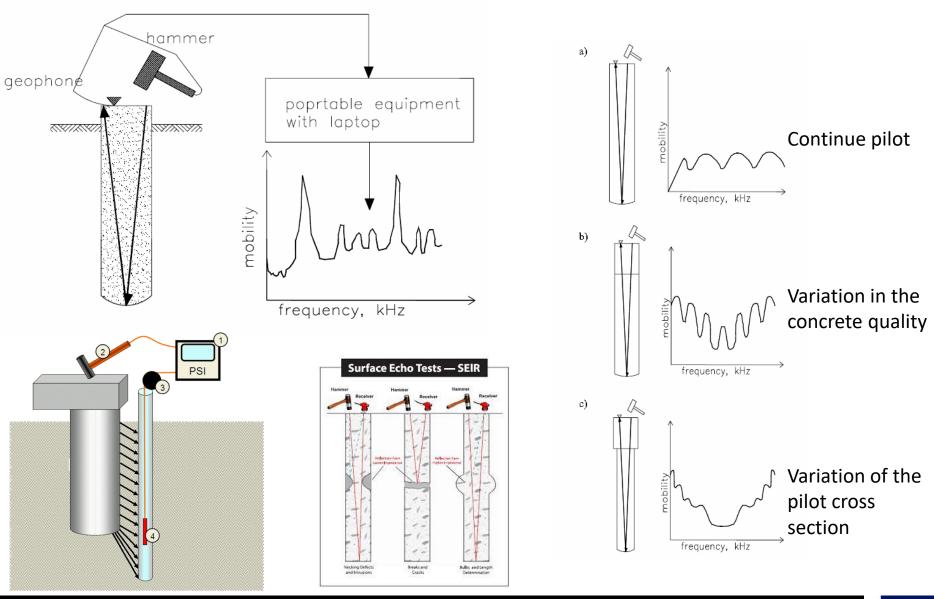


Calcule de volumetrie în cariere





14. INTEGRITY TESTING ON PILOTS

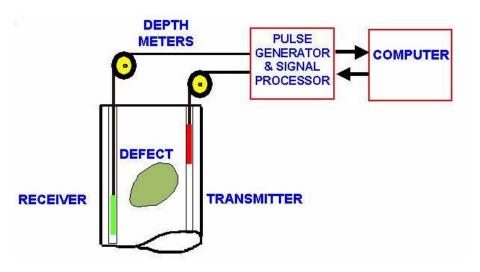


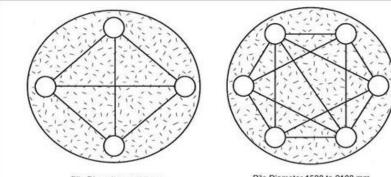
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14. INTEGRITY TESTING ON PILOTS









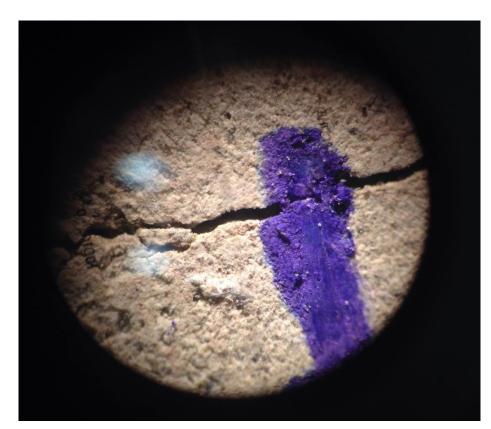
Pile Diameter 1500 to 2100 mm



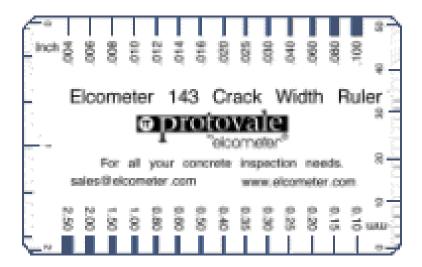


15. OTHER PERFORMANCE AND INTEGRITY TESTS

-CRACK OPENING

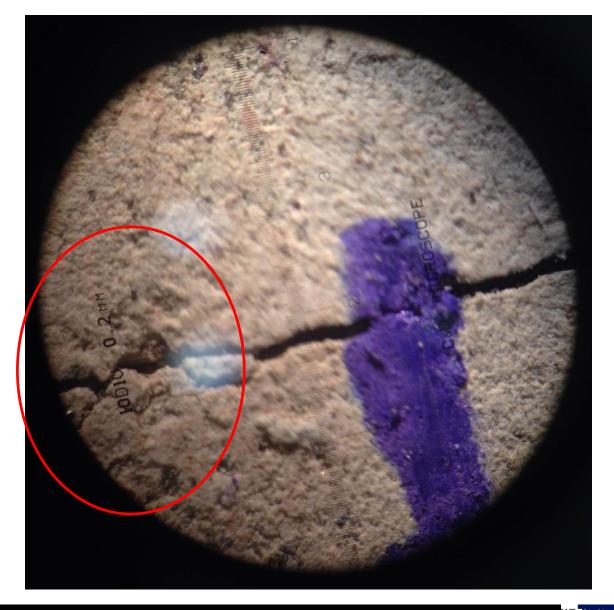




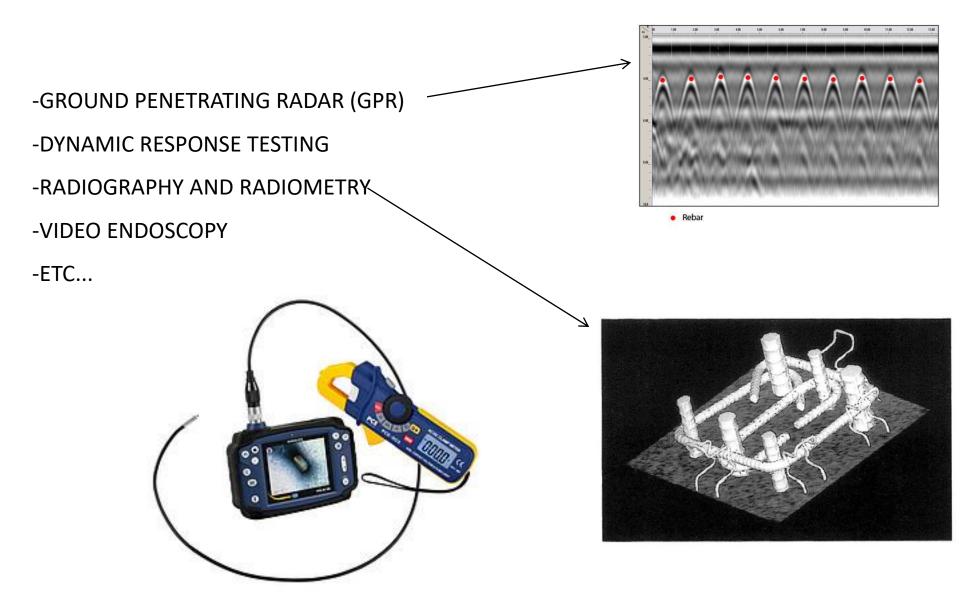


15. OTHER PERFORMANCE AND INTEGRITY TESTS

-CRACK OPENING



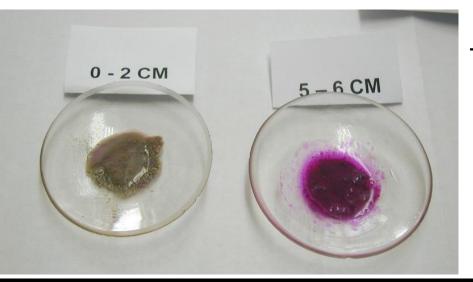
15. OTHER PERFORMANCE AND INTEGRITY TESTS





16. CARBONATION TEST





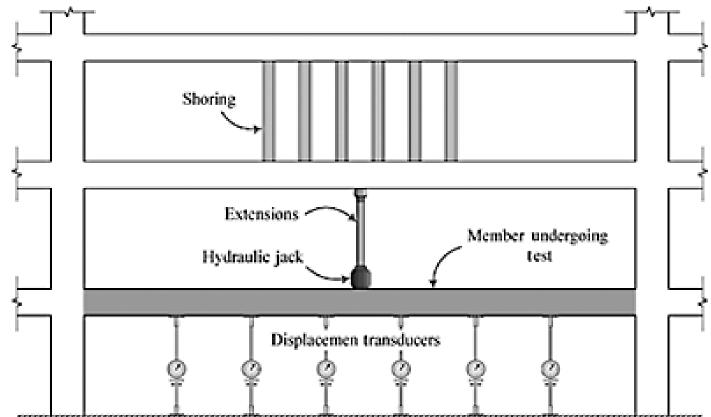
→ Phenolphthalein (indicator in base titrations)
- pink - purplish red → pH > 8...12
- colorless → pH < 8.2



17. REAL SCALE ON-SITE

PUSH-DOWN TEST CONFIGURATION

Setup time:mediuRequirements:hydraulic jack and pump, extensions to ceiling, shoring of above floorLoad variation:EasyReaction:Shored floor(s) above test memberLimitations:Requires floor(s) above for reaction



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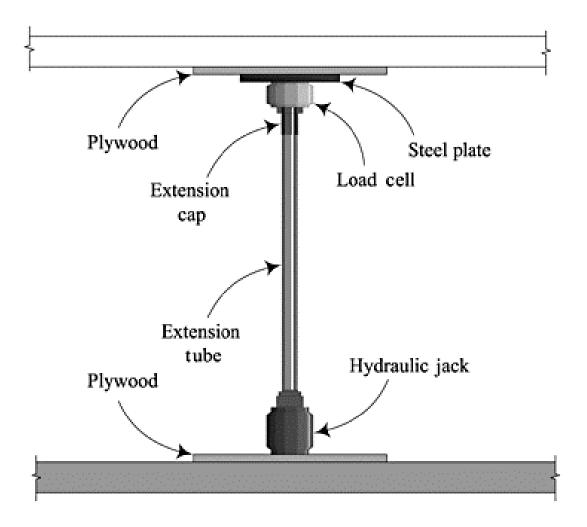
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LOAD TESTING METHODS

(Balaguru, 2009)

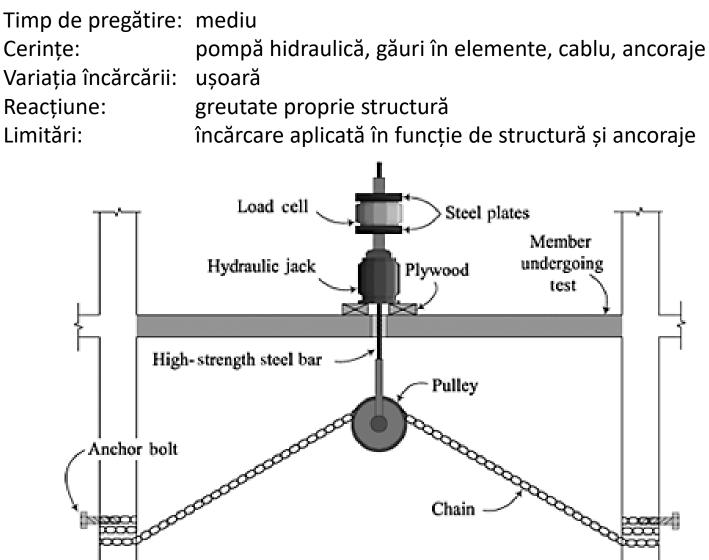
17. REAL SCALE ON-SITE PUSH-DOWN TEST CONFIGURATION





17. REAL SCALE ON-SITE

PULL-DOWN TEST METHOD WITH A FIXED REACTION





17. REAL SCALE ON-SITE

PULL-DOWN TEST CONFIGURATION WITH A MOBILE REACTION

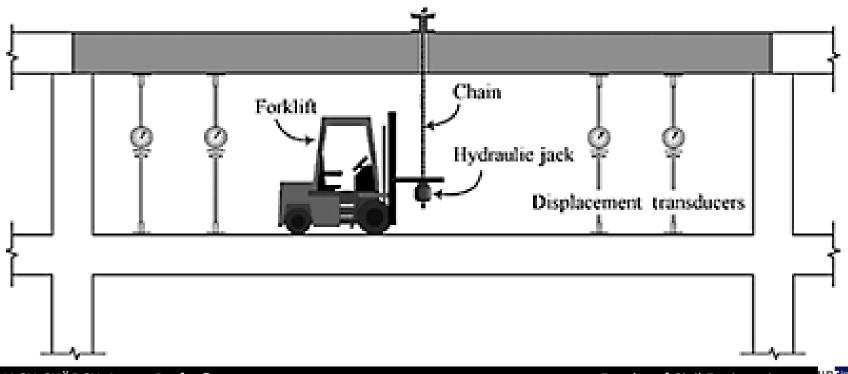
Timp de pregătire: mediu

Cerințe: pompă hidraulică, găuri în elemente, cablu, motostivuitor

Variația încărcării: ușoară

Reacțiune: greutate proprie structură

Limitări: încărcare aplicată relativ mică

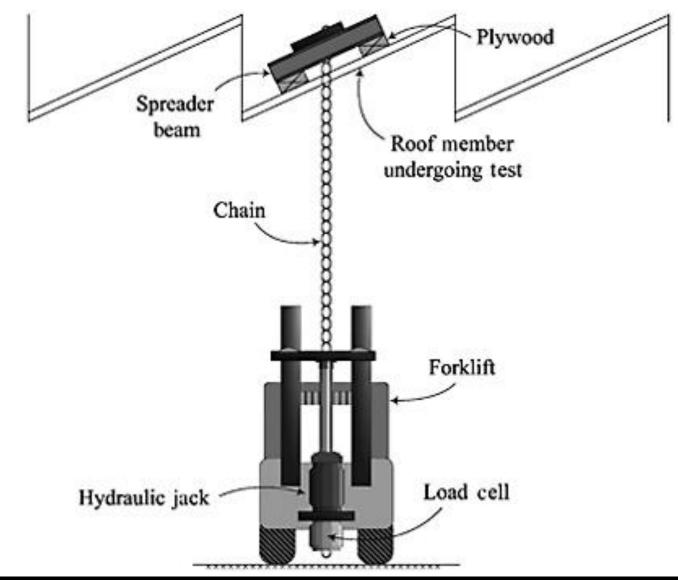


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(Balaguru, 2009)

17. REAL SCALE ON-SITE

PULL-DOWN TEST CONFIGURATION WITH A MOBILE REACTION





17. REAL SCALE ON-SITE CLOSED-LOOP TEST CONFIGURATION

Timp de pregătire: lung

Cerințe:

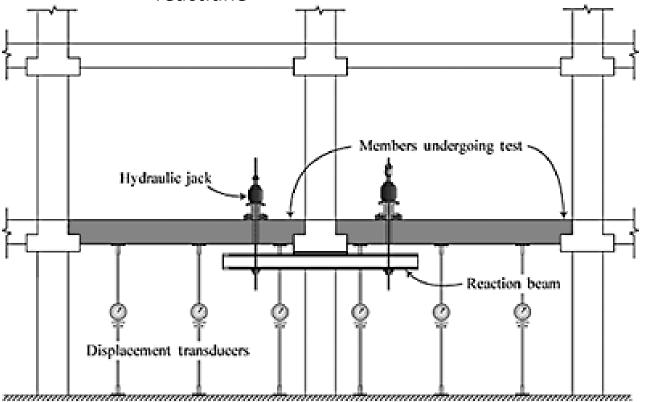
Limitări:

2 pompe hidraulice, găuri în elemente, cabluri, grinzi de reacțiune

Variația încărcării: ușoară

Reacțiune: element intermediar între elementele testate

locația și magnitudinea încărcării în funcție de proprietățile grinzii de reactiune



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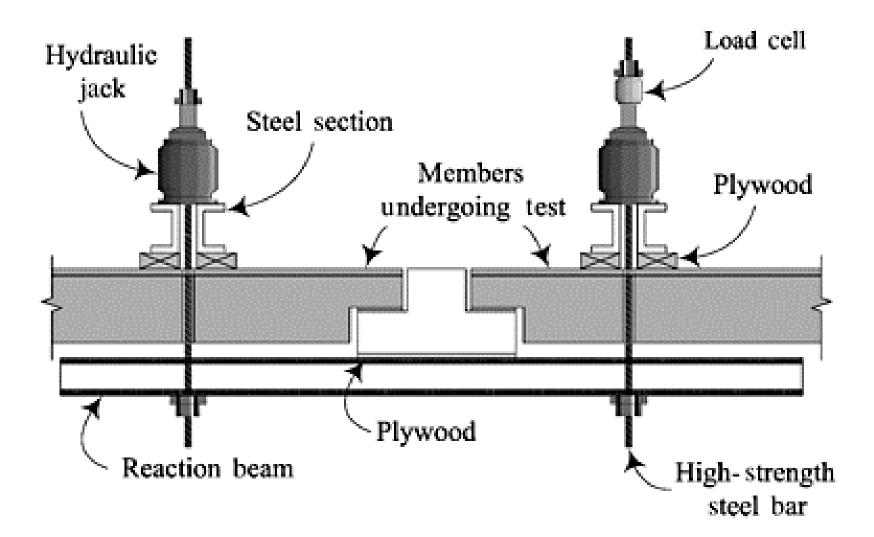
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LOAD TESTING METHODS

(Balaguru, 2009)

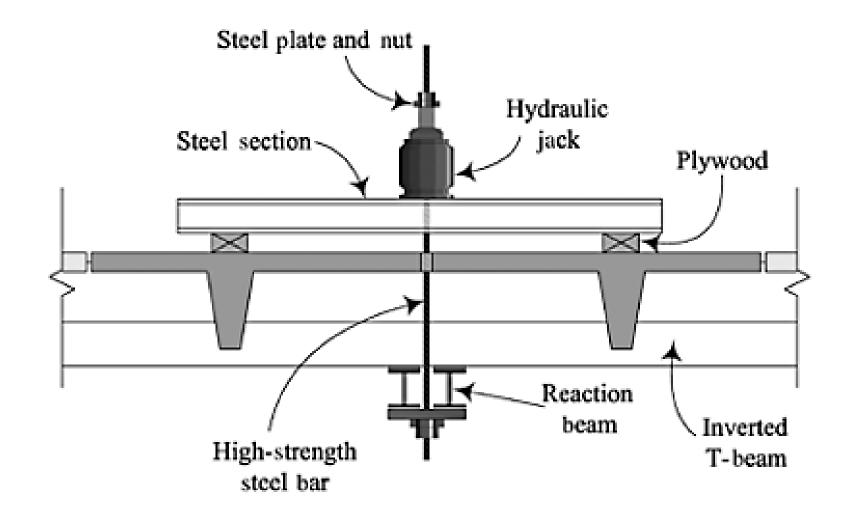
17. REAL SCALE ON-SITE CLOSED-LOOP TEST CONFIGURATION





(Balaguru, 2009)

17. REAL SCALE ON-SITE CLOSED-LOOP TEST CONFIGURATION

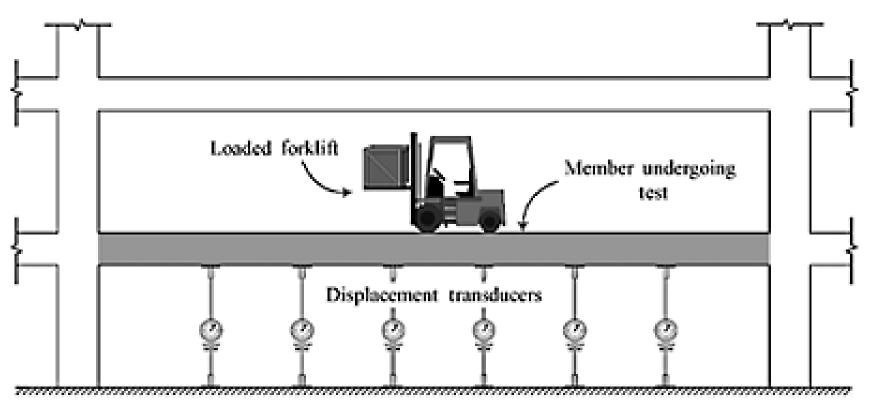




(Balaguru, 2009)

17. REAL SCALE ON-SITE VEHICLE-LOADED TEST CONFIGURATION

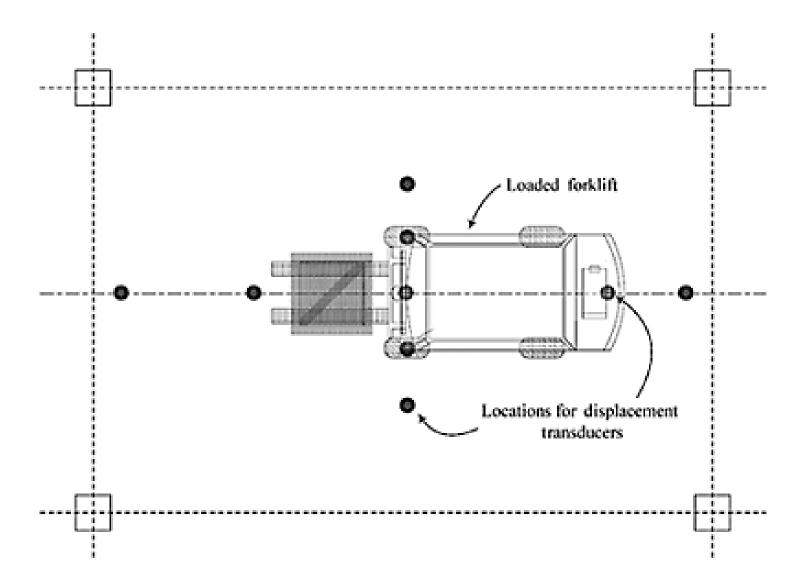
Timp de pregătire : scurtCerințe:vehicule, greutățiVariația încărcării:cu dificultateReacțiune:nu se aplicăLimitări:variația încărcării este îndelungată



LOAD TESTING METHODS

(Balaguru, 2009)

17. REAL SCALE ON-SITE VEHICLE-LOADED TEST CONFIGURATION

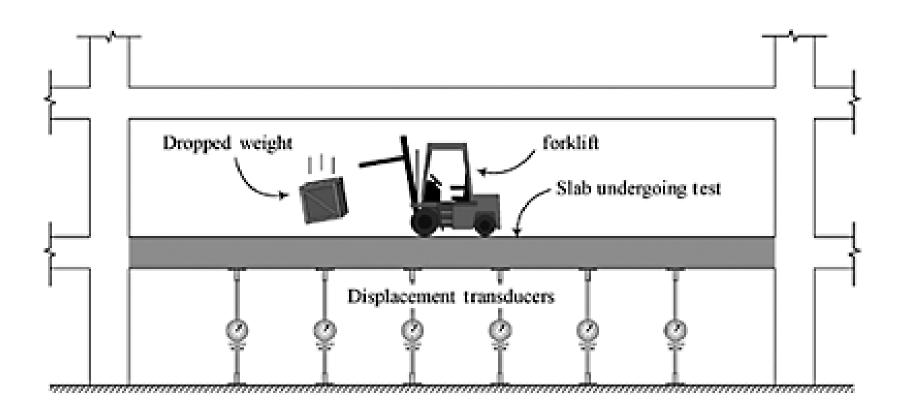




(Balaguru, 2009)

17. REAL SCALE ON-SITE

LOAD TEST CONFIGURATION FOR A DROPPED WEIGHT





17. REAL SCALE ON-SITE VEHICLE-LOADED TEST CONFIGURATION → BRIDGES



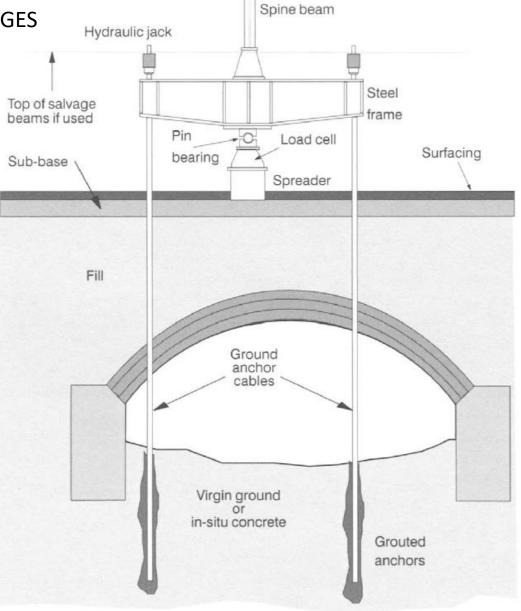




LOAD TESTING METHODS

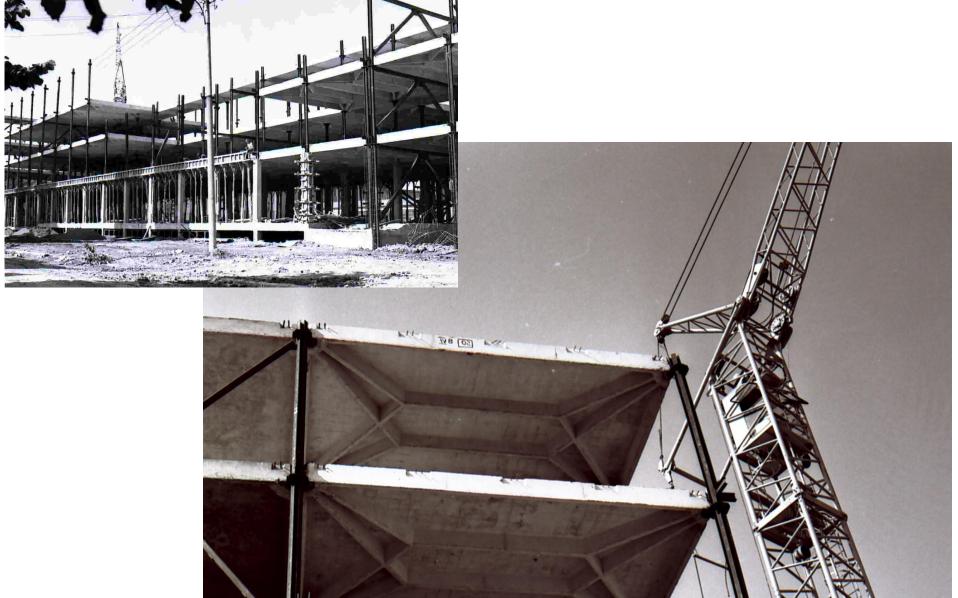
(Ryall M.J.)

17. REAL SCALE ON-SITE LOAD TEST CONFIGURATION → BRIDGES



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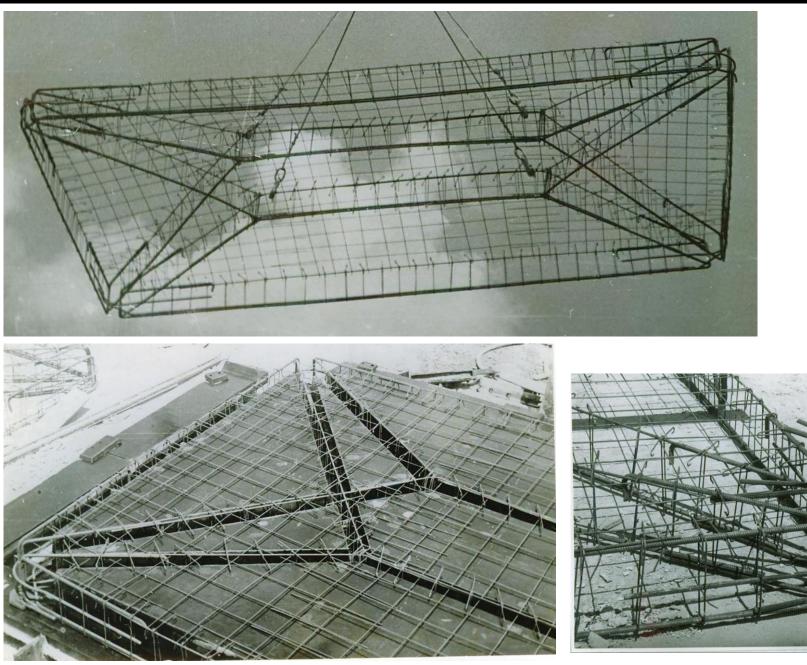






RESEARCH AND DESIGN ASSISTED BY TESTING

LOAD TESTING METHODS



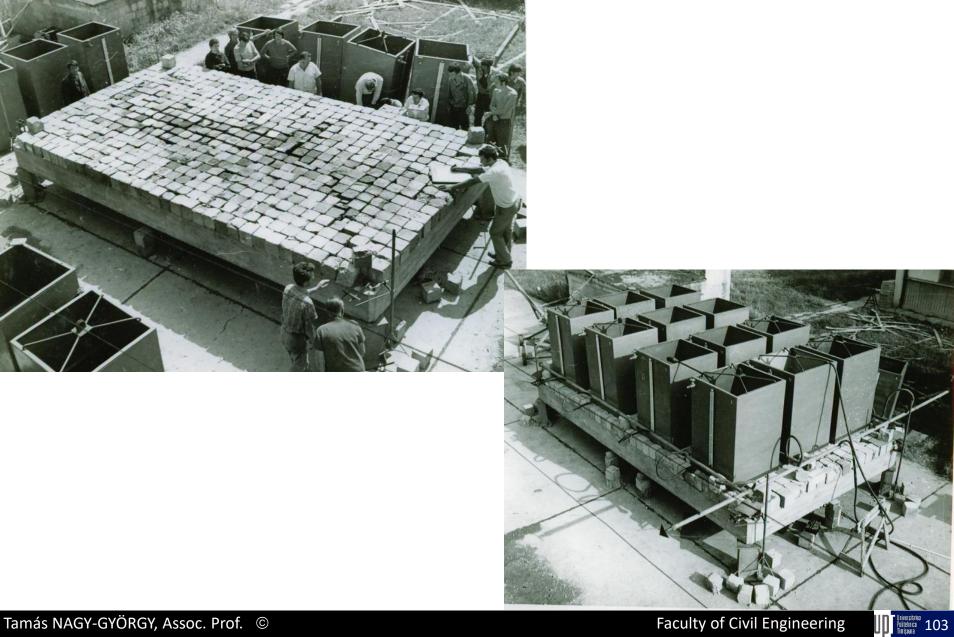


17. REAL SCALE ON-SITE





17. REAL SCALE ON-SITE



17. REAL SCALE ON-SITE LOAD TEST CONFIGURATION → BRIDGES



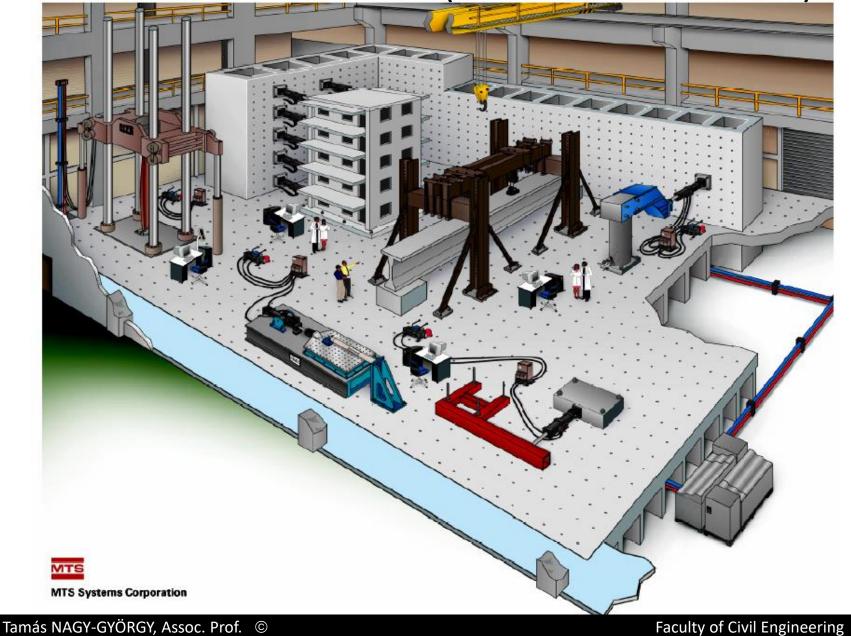








18. LOAD TESTING IN RESEARCH LABS (FULL SCALE OR REDUCED SCALE)

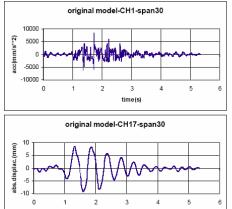


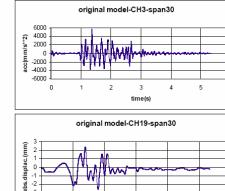






Mustafa Pasha mosque constructed in scale 1:6



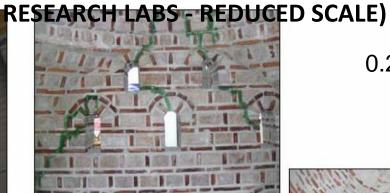


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0.2g ÷1.5g

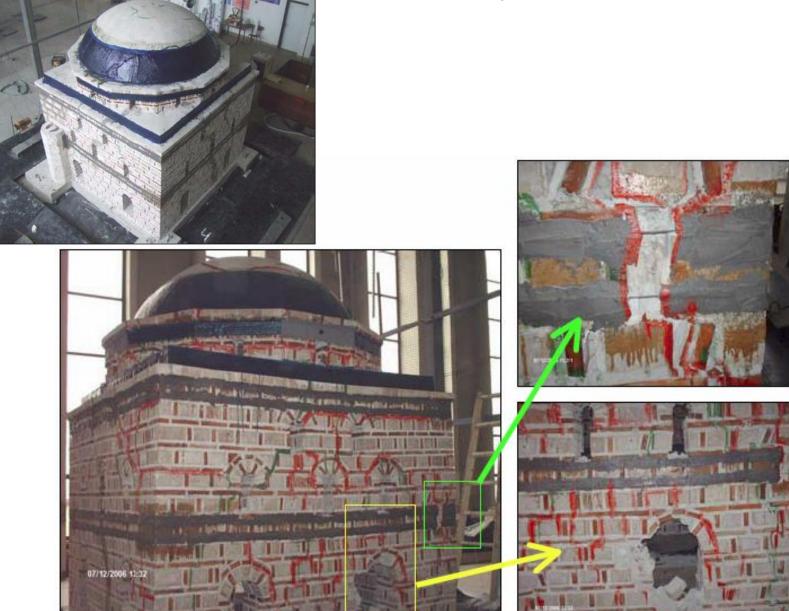




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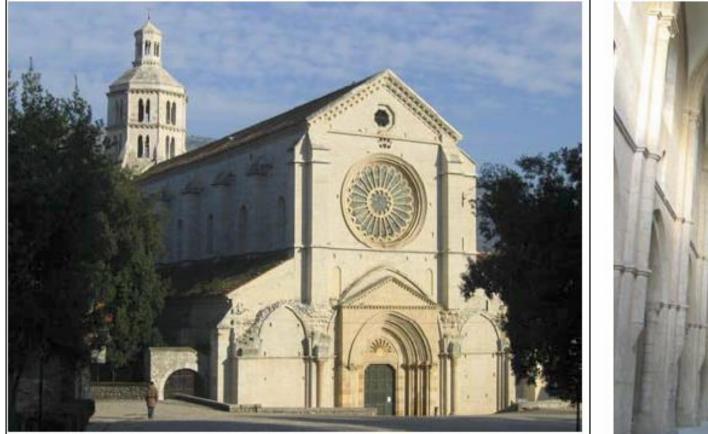
Faculty of Civil Engineering

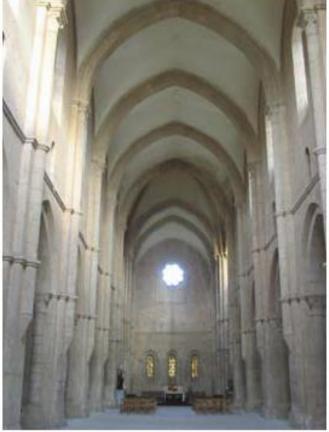
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LOAD TESTING METHODS

(Mazzolani)

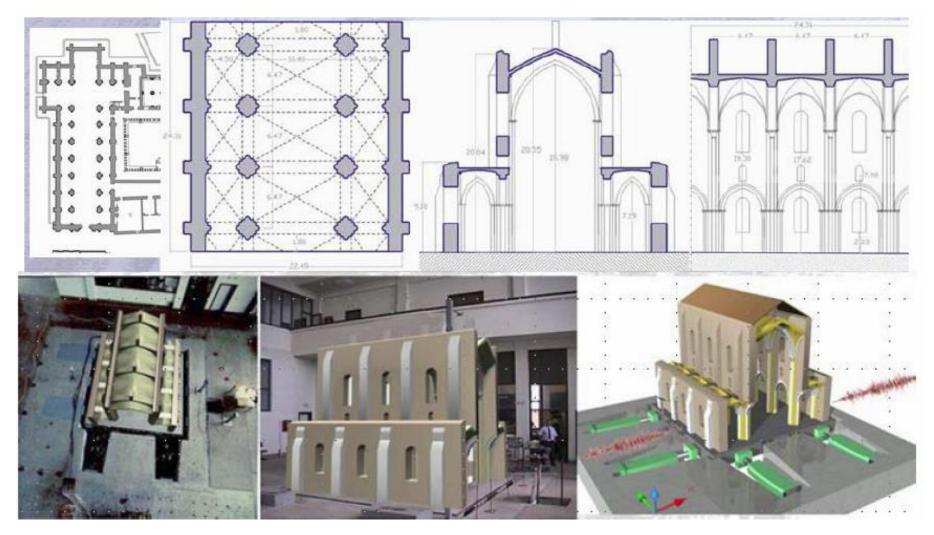
18. LOAD TESTING IN RESEARCH LABS - REDUCED SCALE)





Catedrala Fossanova

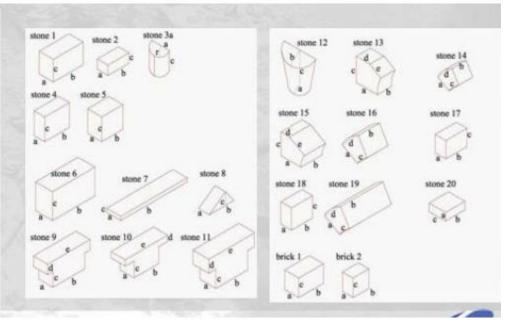


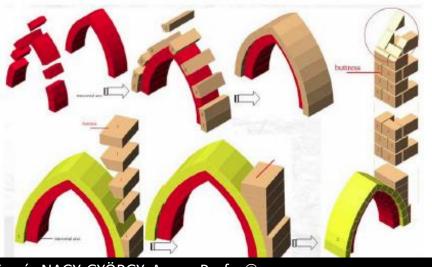


Scara 1: 5.5 Tamás NAGY-GYÖRGY, Assoc. Prof. ©



element	color	a cm	b cm	c cm	d cm	e cm	r cm	n" element
stone 2		6	12	5				11712
stone 3a		10		9			5	512
stone 4		6	14	12				64
stone 5		8	12	12		•		128
stone 6		10	24	14				96
stone 7		8	37	2	+			24
stone 8	5	6	14	6.5				140
stone 9		8	18	7.5	5	24		160
stone 10		8	14	8	3	24		146
stone 11		9	18	9	6.5	26		120
stone 12		6	9	16.5	-			8
stone 13		12	8	12.85	7.7	8.7		32
stone 14		7.3	8	5.5	8.2			40
stone 15		11.35	12	11.5	4.8	9.8		32
stone 16		9.8	12	7.8	12.9			40
stone 17	1	6	12	9				2328
stone 18		5.7	11.35	14.4		+		240
stone 19		6	6.8	24	-			48
stone 20		9	4	11	-			278
brick 1	1	8	14	10				56
brick 2		6	9	10	+			112









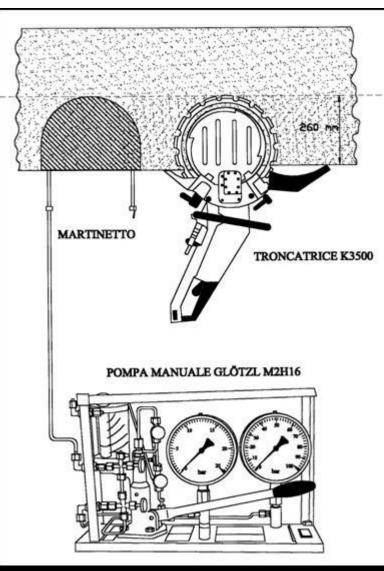




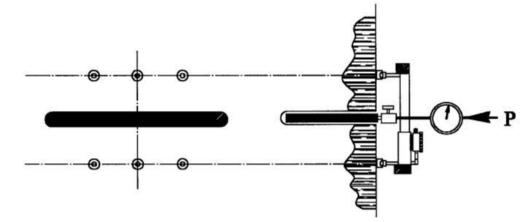
TEST ON MASONRY

(Boviar)

19. TEST ON MASONRY USING FLAT JACK







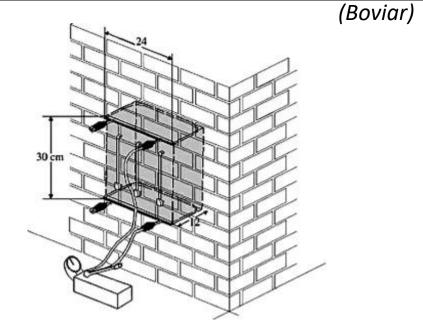


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TEST ON MASONRY

19. TEST ON MASONRY USING FLAT JACK FLAT JACKS → 3500 KN











19. TEST ON MASONRY USING SCHMIDT OS-120 PENDULUM HAMMERS

 \rightarrow

lightweight concrete (LC) gypsum boards mortars in masonry joints





TEST ON METALS

(Proceq)

20. TEST ON METALS USING ECQUOTIP

Hardness Testers

- Brinell
- Rockwell
- Shore
- Vickers
- Rockwell C





RESEARCH AND DESIGN ASSISTED BY TESTING

lecture notes -

NAGY-GYÖRGY Tamás

Professor

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Tel: +40 256 403 935

Web: <u>http://www.ct.upt.ro/users/TamasNagyGyorgy/index.htm</u>

Office: A219

THANK YOU FOR YOU ATTENTION!

